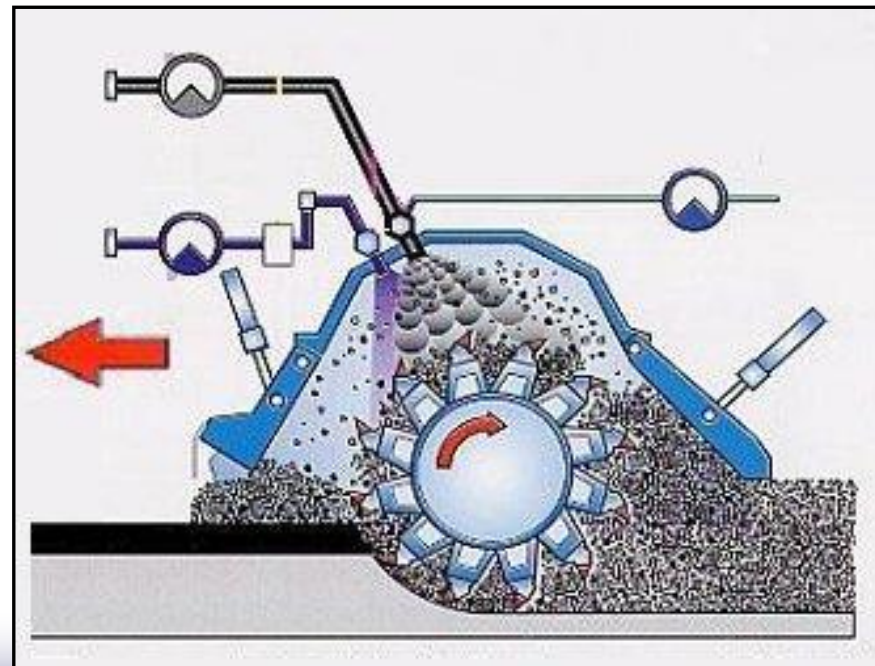




INNOVATION IN COLD RECYCLING TECHNOLOGIES

COLD RECYCLING AND LIME



ACKNOWLEDGEMENTS



University of Parma

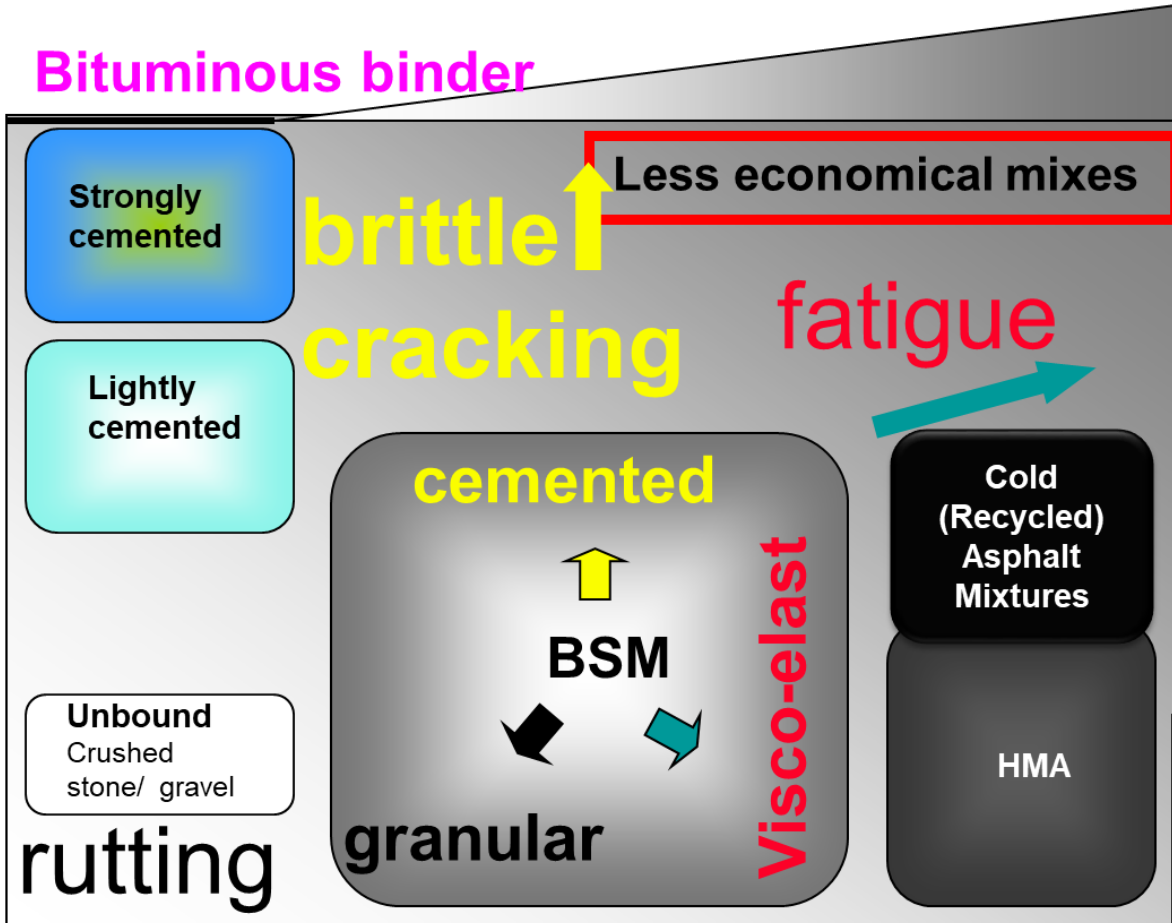
- **Kim Jenkins**
- **Gordon Airey**
- **Alessandro Marradi**
- **Martin Diekmann**
- **Elena Romeo**

BACKGROUND

“Linear-elastic”

Bituminous binder

6% 4%



brittle cracking

fatigue

cmented

BSM

Visco-elast

Cold (Recycled) Asphalt Mixtures

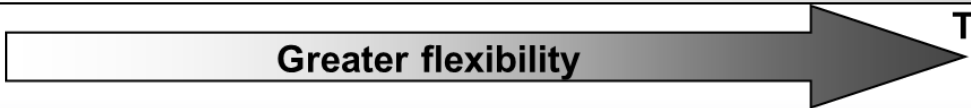
HMA

rutting

Active Filler

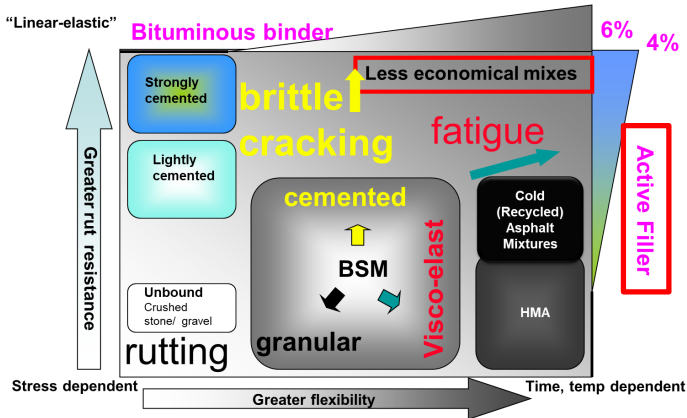
Stress dependent

Time, temp dependent



Courtesy of Kim Jenkins

BACKGROUND

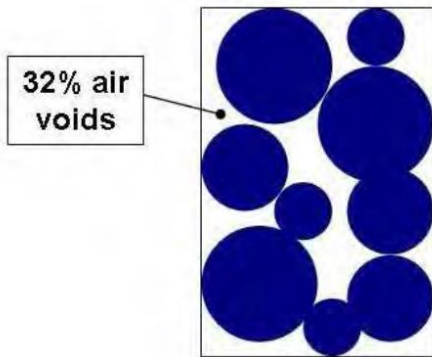


i Active filler

The term active filler is used to define fillers that **chemically alter** the mix properties. This includes fillers such as lime, cement and fly ash but excludes natural fillers such as rock flour.

The purpose of incorporating active filler in BSM is to:

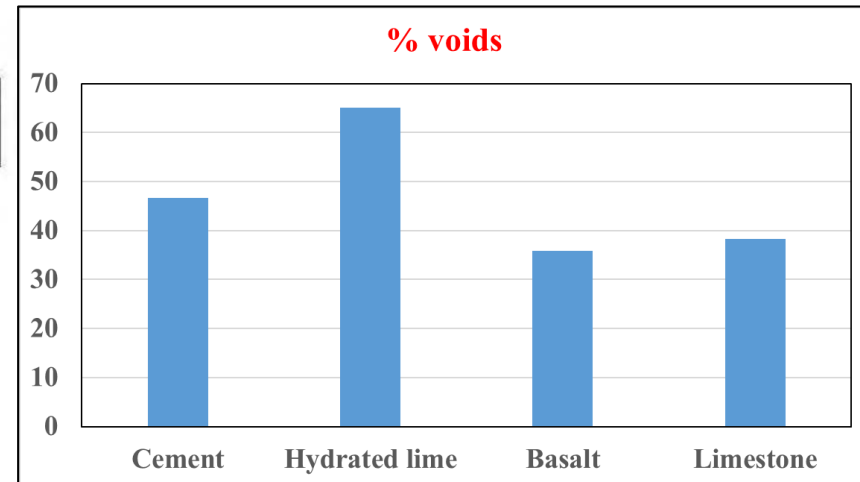
- Improve adhesion of the bitumen to the aggregate
- Improve dispersion of the bitumen in the mix
- *Modify the plasticity of the natural materials*
- Increase stiffness & strength of mix
- Accelerate curing of compacted mix
- Control emulsion’s breaking time



mineral filler



hydrated lime

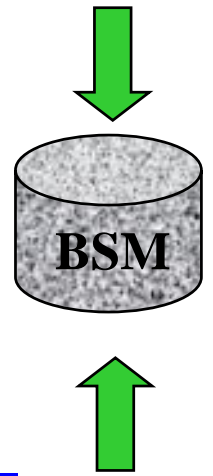
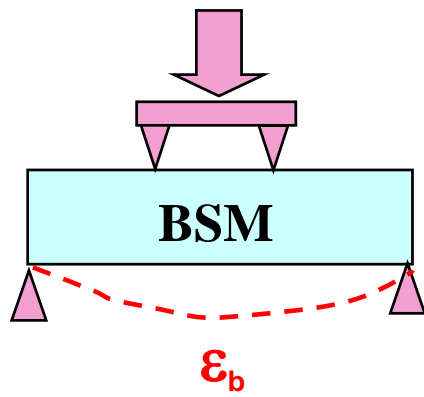


BACKGROUND



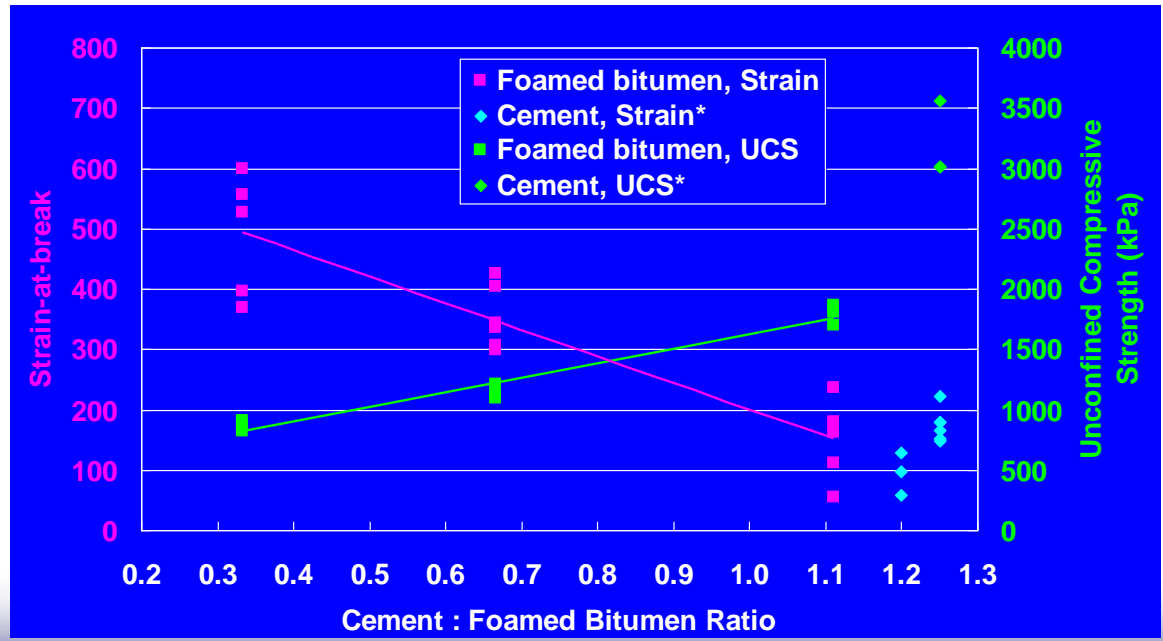
Cement Contents

The cement content of BSMs should be $\leq 1\%$, and the cement content should not exceed the bitumen content.



..... $\rightarrow \leq 1,5\%$

Strength and flexibility


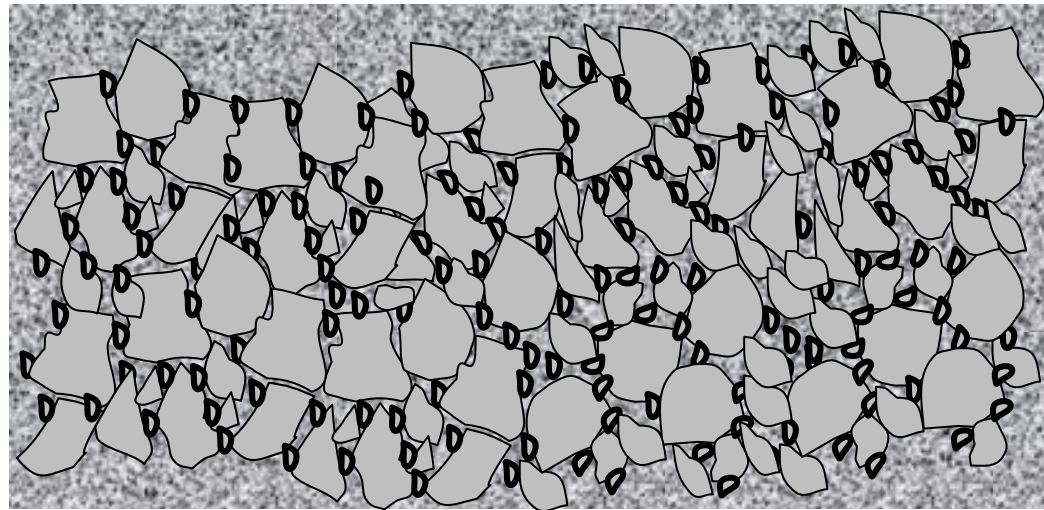


Courtesy of Kim Jenkins

BACKGROUND

A Bitumen Stabilized Material is an half bounded material: a material with a behavior “at half way” between a granular material and a bounded material

“..a granular material with steroids.....” [Dave Collings]



Cement Contents

The cement content of BSMs should be $\leq 1\%$, and the cement content should not exceed the bitumen content.

..... $\rightarrow \leq 1,5\%$

...with high amount of cement the Bitumen Stabilized Material loses the characteristics of a granular material: it becomes a Cement Bound Material

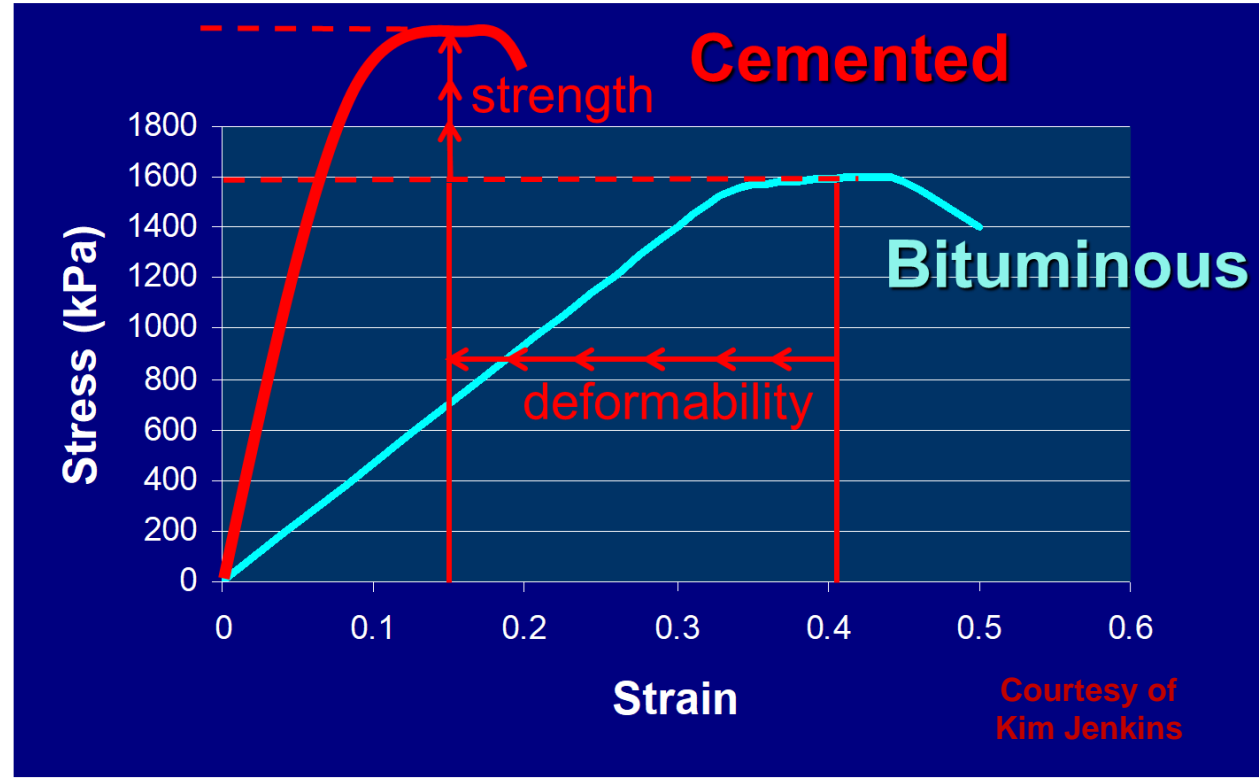
BACKGROUND



Cement Contents

The cement content of BSMs should be $\leq 1\%$, and the cement content should not exceed the bitumen content.

..... $\rightarrow \leq 1,5\%$



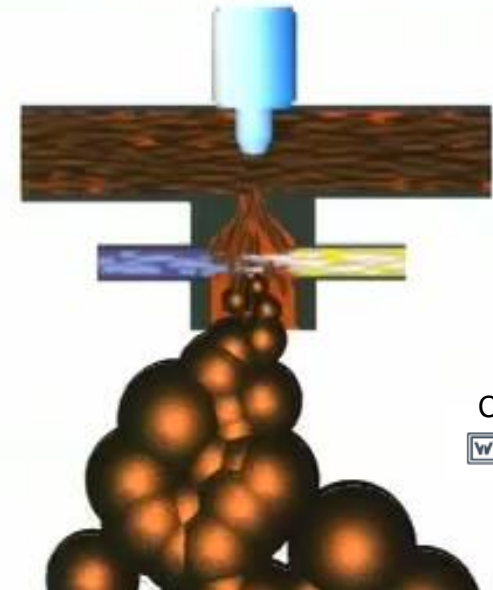
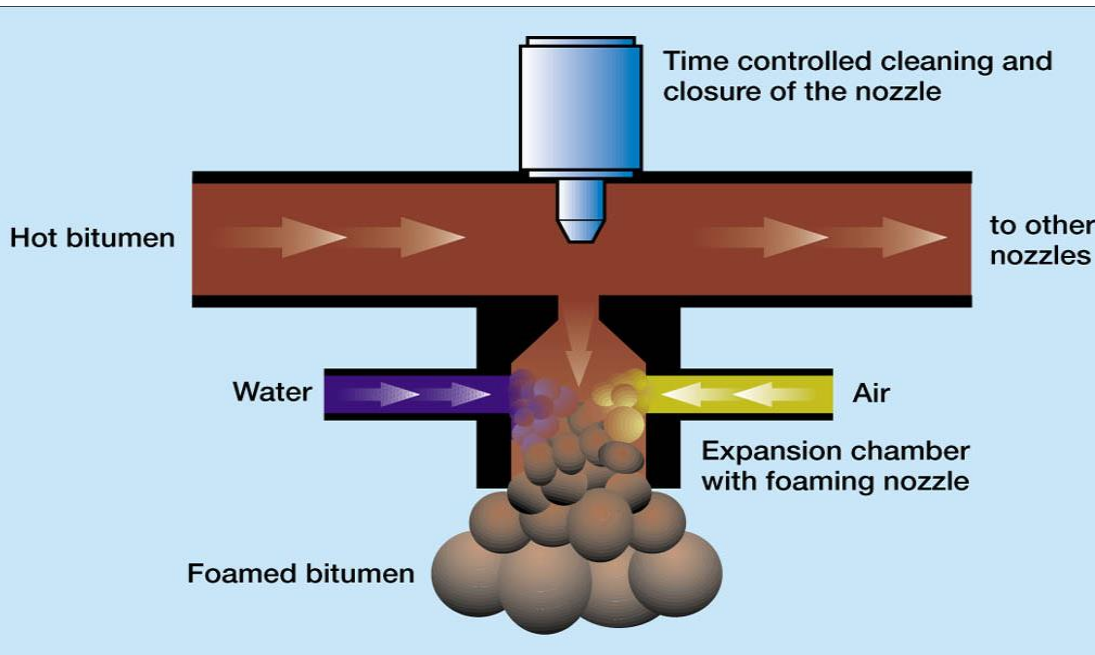
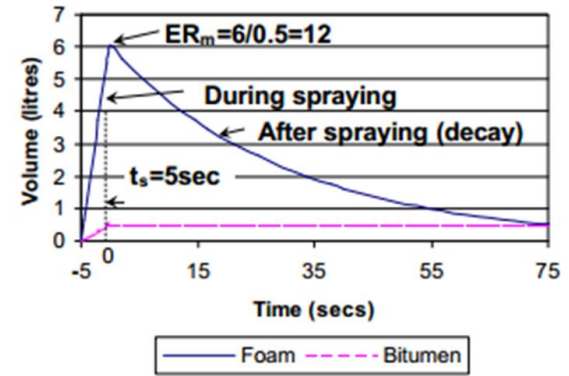
...with high amount of cement the Bitumen Stabilized Material loses the characteristics of a granular material: it becomes a Cement Bound Material

BACKGROUND

Foam bitumen

Cold water and air are injected simultaneously into the hot asphalt.

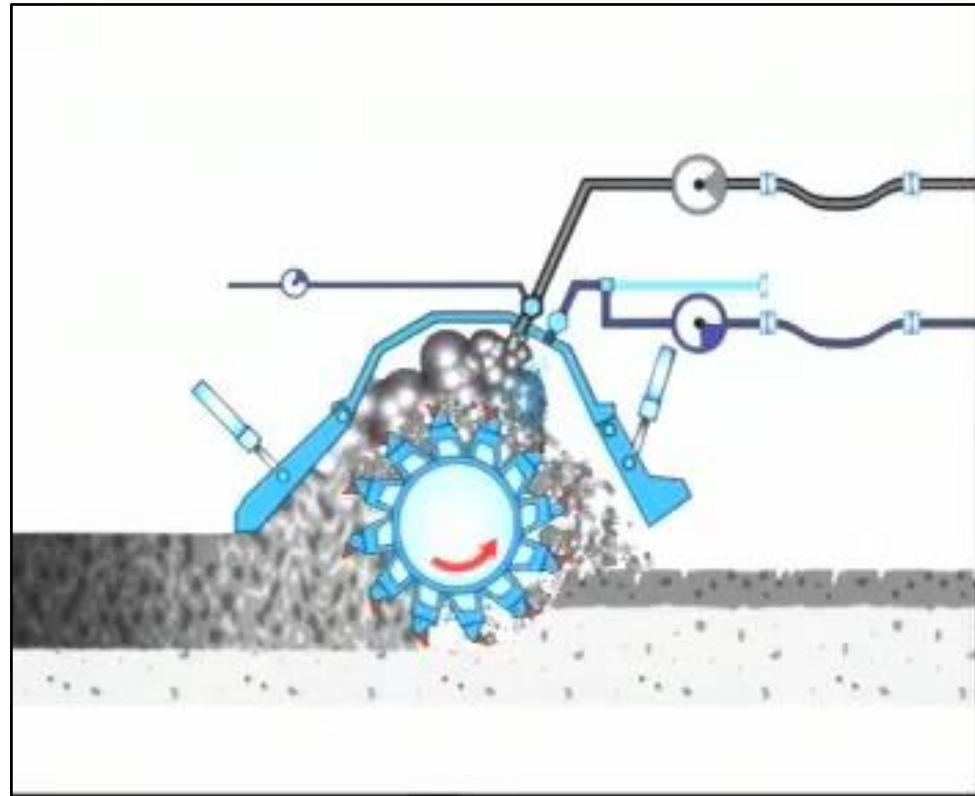
The hot asphalt foams explosively and shoots down into the mixing chamber.



Courtesy of
 WIRTGEN

BACKGROUND

Foam bitumen



Courtesy of



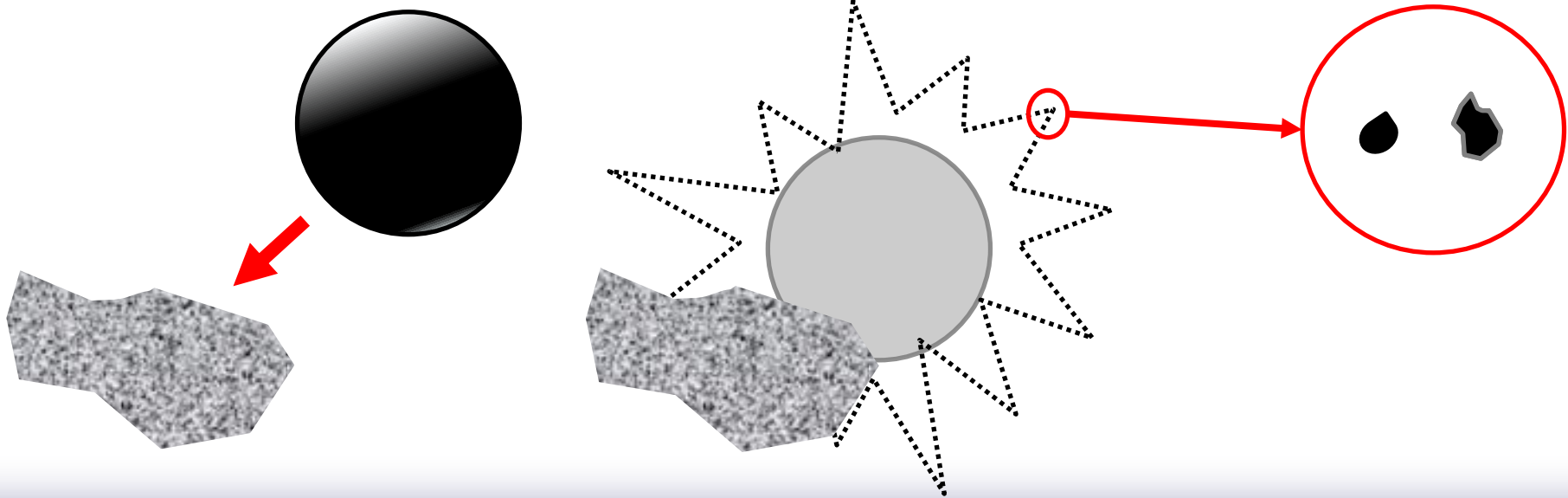
BACKGROUND

Foam bitumen

Foamed bitumen is a mass of bubbles. Each bubble is a thin (very thin) film of bitumen surrounding steam (water vapor)

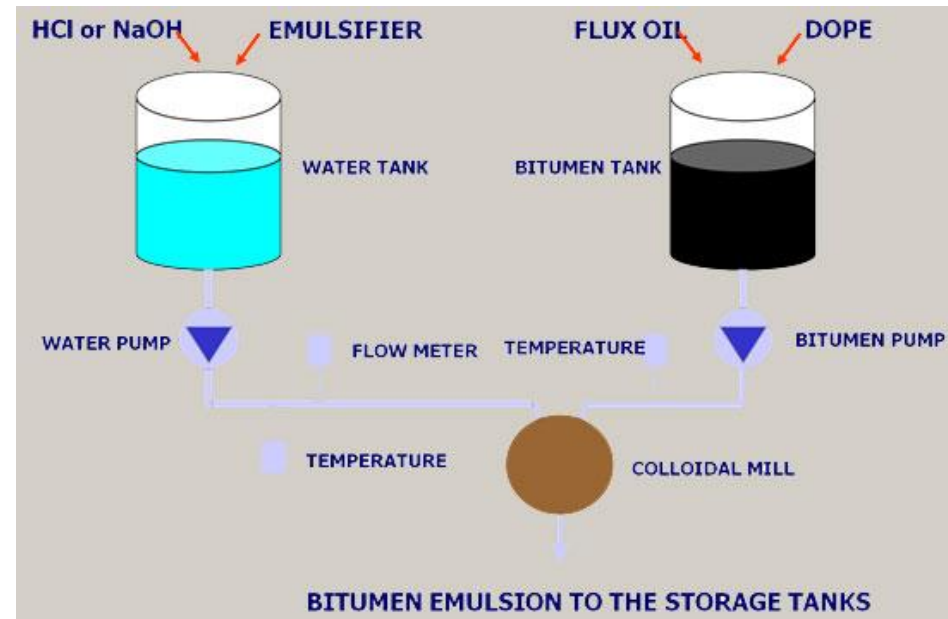
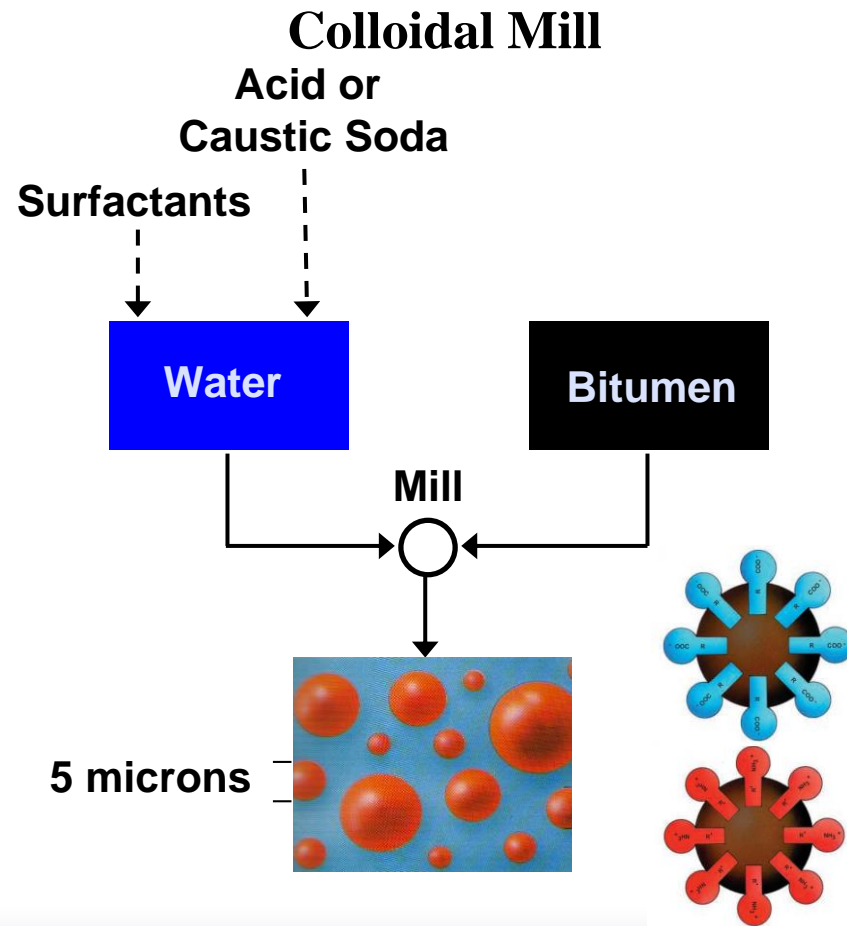
Because the film of bitumen is so thin, if a stone is thrown at one of these bubbles, it will shatter into thousands (maybe millions?) of tiny bitumen splinters. Each bitumen splinter has only sufficient heat energy to warm a dust particle (+moisture) and adhere to it (or attach itself onto another bitumen splinter)

The blasting and the dust particles are the carriers of bitumen



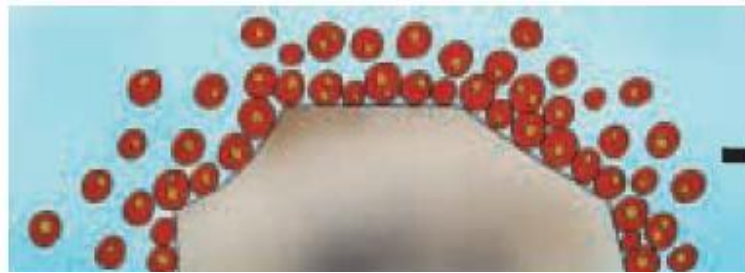
BACKGROUND

Bituminous emulsion



BACKGROUND

Bituminous emulsion



decantation



flocculation



coagulation



setting

BACKGROUND



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Bituminous emulsion

Bitumen emulsion is a form of paint, so it “wet” the surface of all particles of the mixture of aggregates

The water is the carrier of bitumen



Courtesy of Kim Jenkins

BACKGROUND



University of Parma

Bituminous emulsion

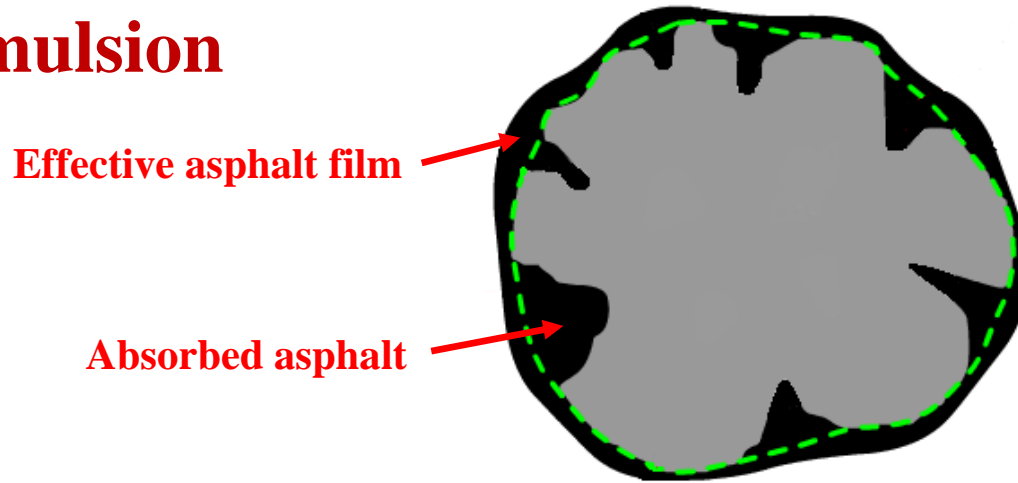
Because the surface area of the fines aggregates it is much bigger than the surface area of coarse aggregates, the emulsion will be concentrated on the fines fraction.

Particle size (mm)	Surface area factor (m ² /kg)
19	0.13
13.2	0.18
9.5	0.24
6.7	0.31
4.75	0.43
2.36	0.82
1.18	1.64
0.6	2.87
0.3	6.14
0.15	12.24
0.075	32.77

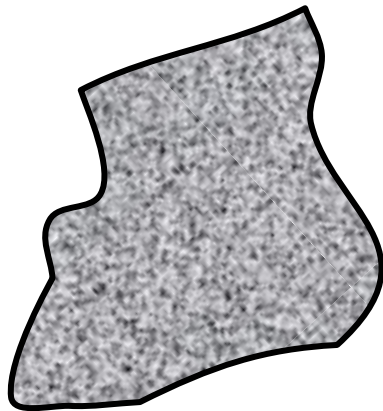
252 times higher

BACKGROUND

Bituminous emulsion



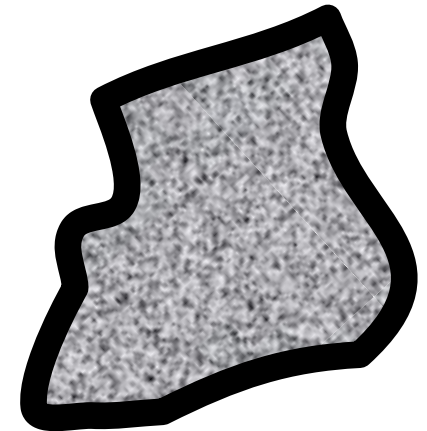
Non-continuously bounded material



**Bituminous emulsion
stabilized material**

A bituminous emulsion stabilized materials has the aggregates fully covered by the emulsions, but the film made by the emulsion (in total around 2-3% of the weight of aggregates) it is not enough thick to make a fully bounded material

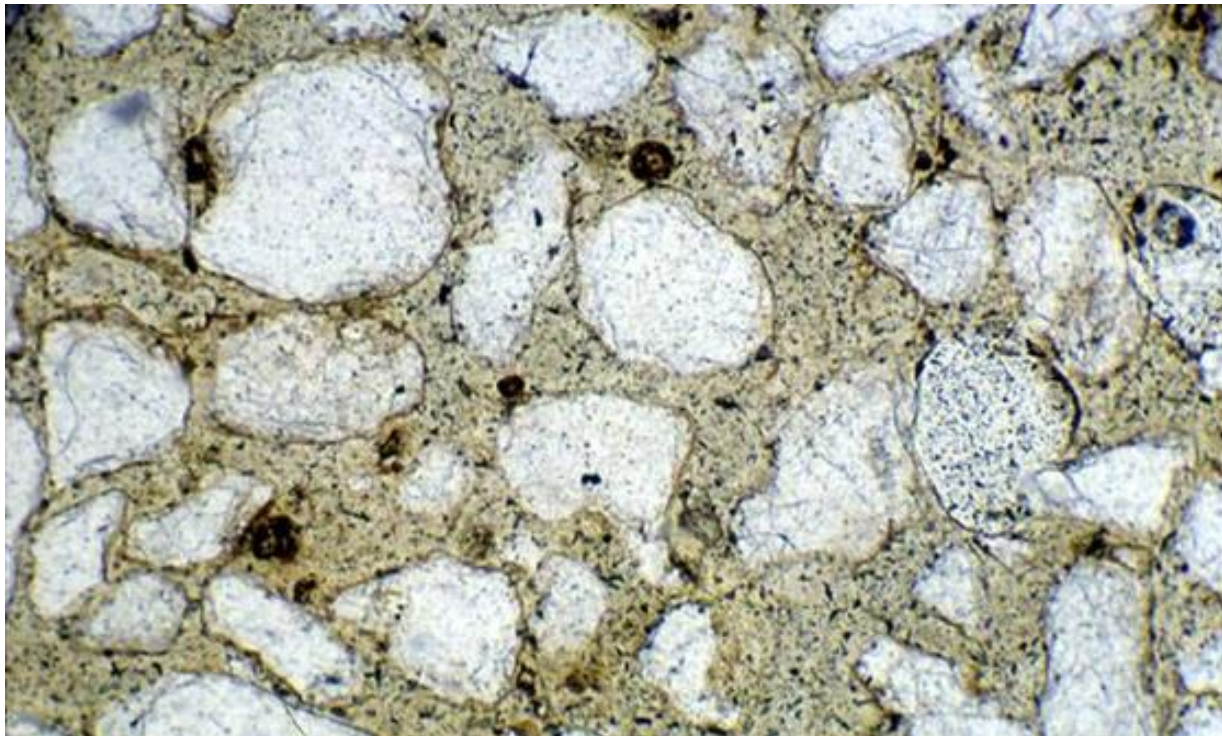
Continuously-bounded material



Hot (cold) asphalt mixture

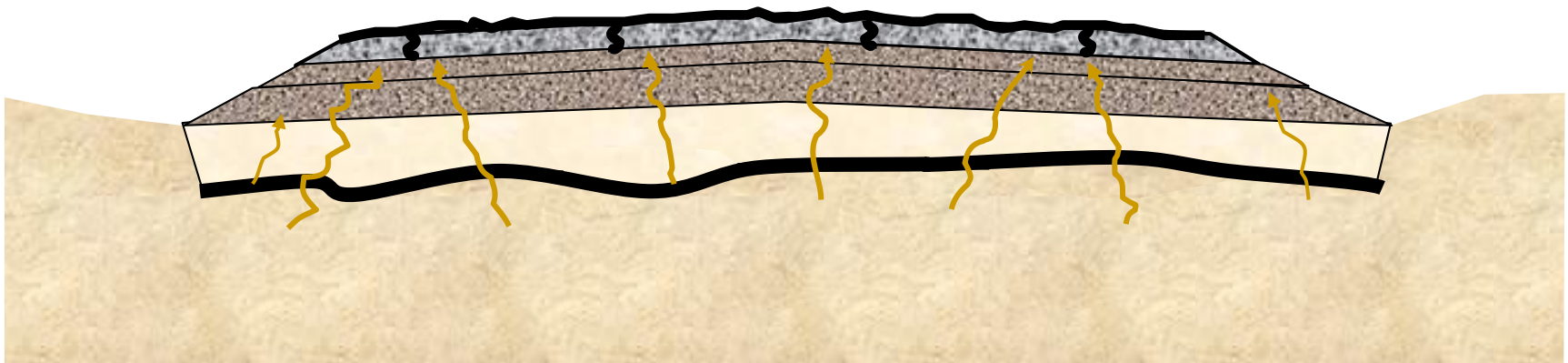
BSMs are non-continuously bounded materials

The bitumen carried by fine aggregates (mainly by filler) it makes an adhesive mastic disperse inside the mixture



PROBLEM STATEMENT

Rising of clay particles:
it requires to join lime stabilization and cold in place recycling



PROBLEM STATEMENT

Can we use the lime as active filler to have only one lay down phase?

Can make sense a «combined lime-bitumen stabilization»?

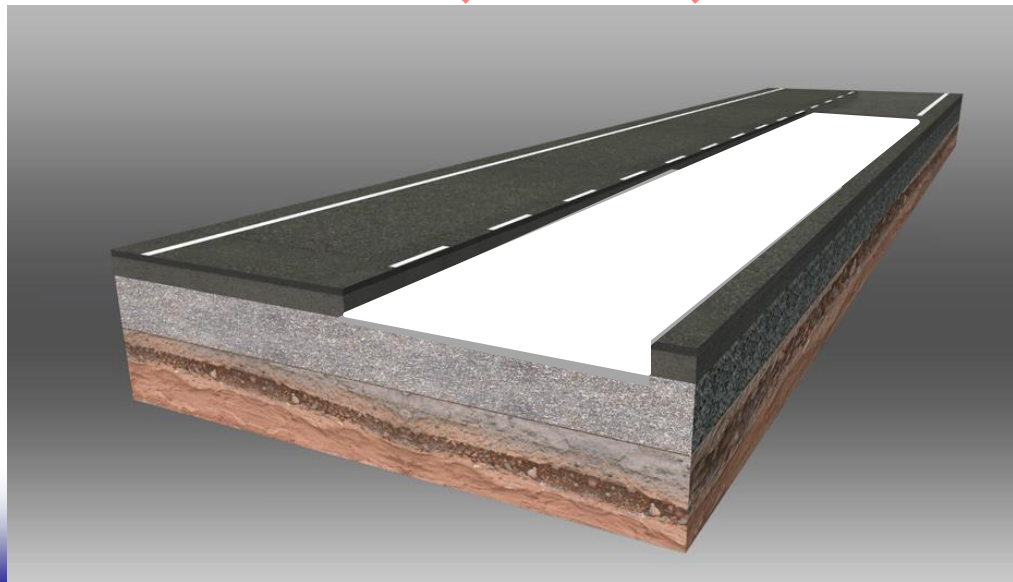
1. Lime to stabilize the clay



2. Cement as active filler

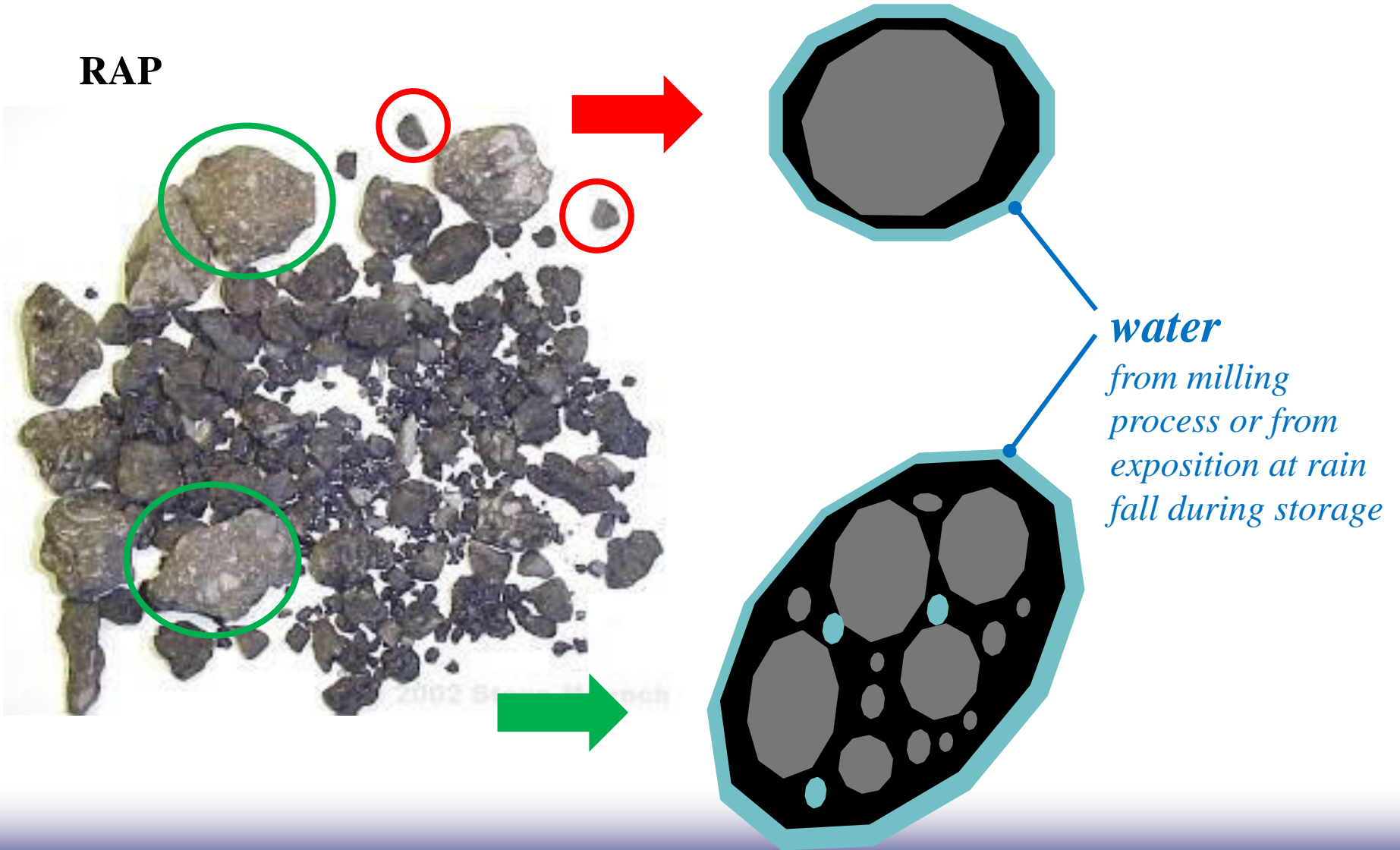


3. mixing, foaming,



PROBLEM STATEMENT

RAP

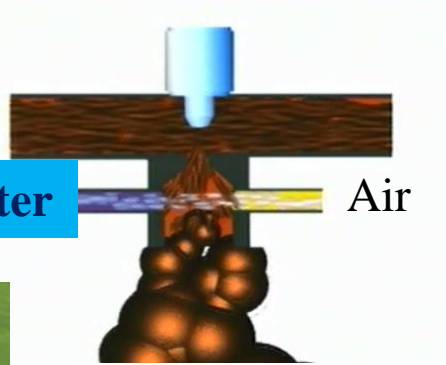


PROBLEM STATEMENT

Bitumen

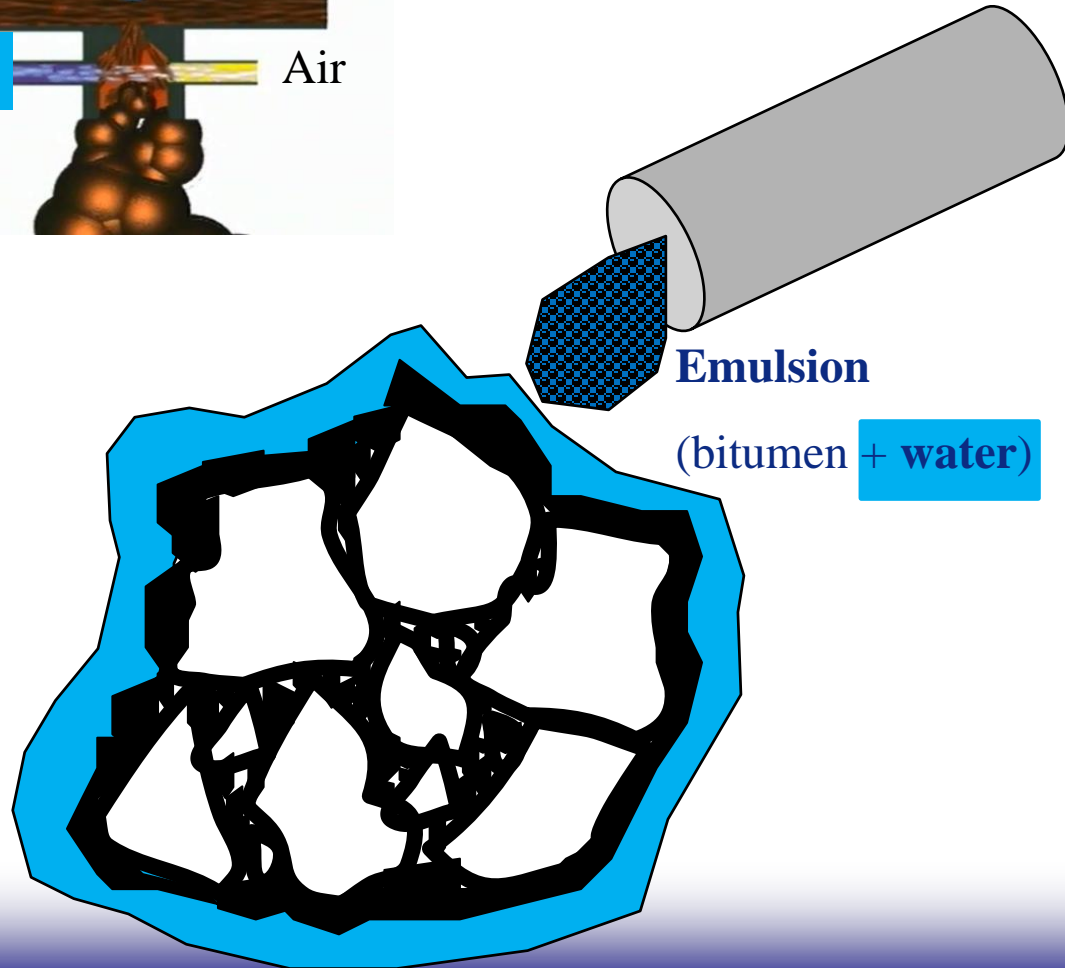
Water

Air

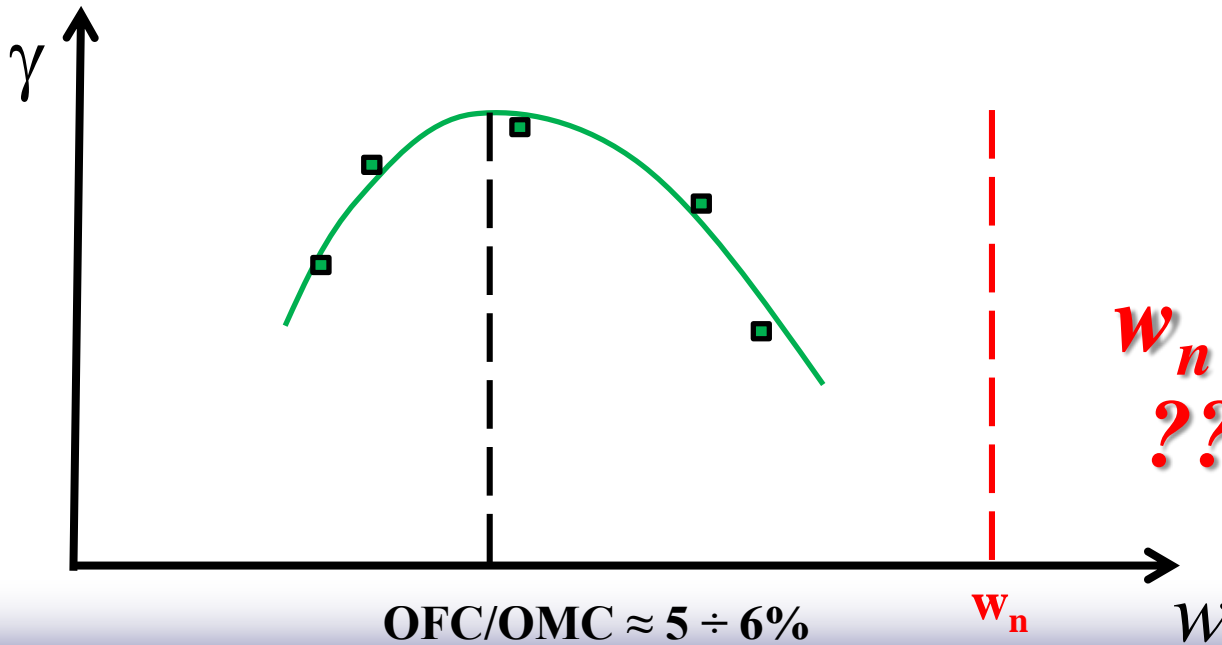
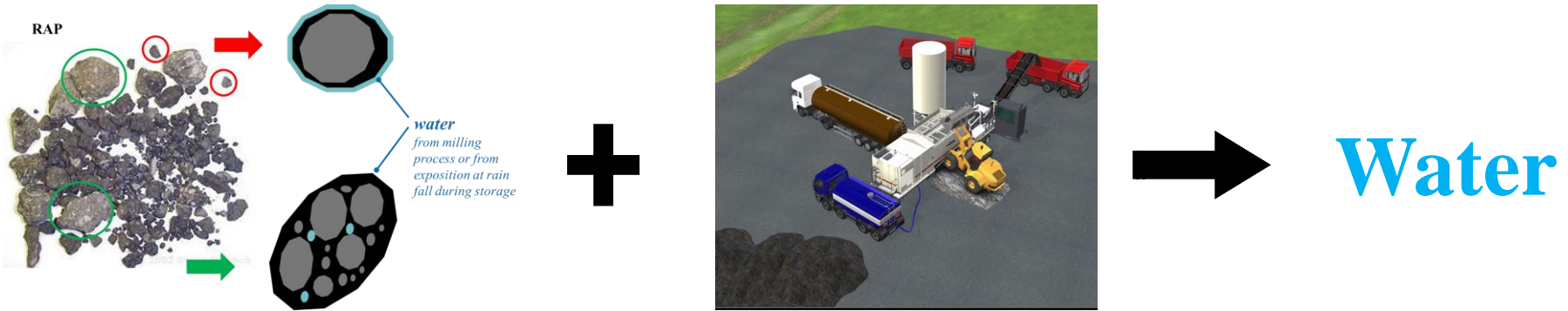


Emulsion

(bitumen + water)



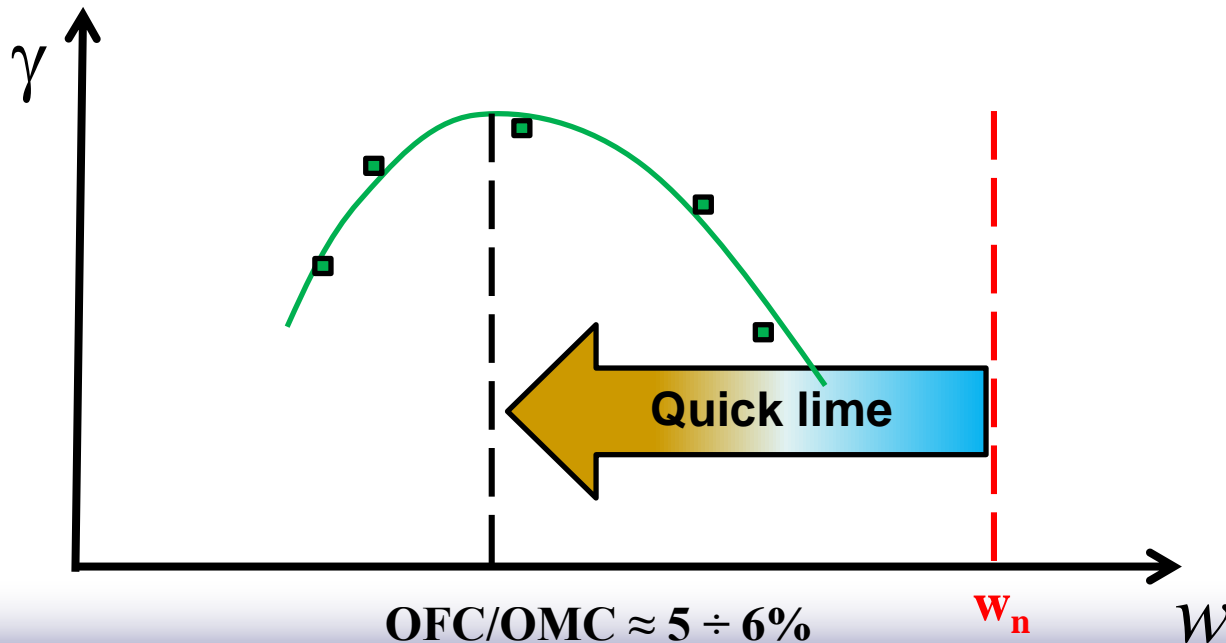
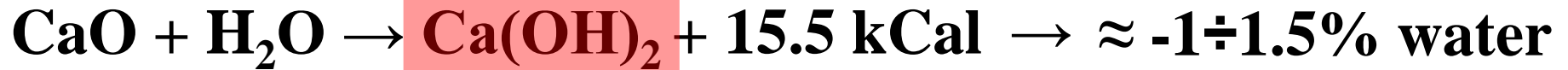
PROBLEM STATEMENT



$w_n \gg OFC$
??????????

PROBLEM STATEMENT

Can we use this hydrated lime as active filler?



PROBLEM STATEMENT



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- **Can the lime be used instead of cement?**
- **Can the lime partially replace the cement in the total amount of active fillers?**

OBJECTIVE AND SCOPE



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- **Evaluate the influence on bearing capacity of introducing lime in bitumen emulsion recycled mixtures**
- **Evaluate the influence on bearing capacity of introducing lime in foam bitumen recycled mixtures**
- **Evaluate the influence on bearing capacity of use of lime instead of cement in foam bitumen recycled mixtures**

BINDERS CHARACTERISTICS



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Emulsion

Over stabilized emulsion

→ *1% it is the minimum amount of cement to start the breaking process*

Bitumen content: 60%

Bitumen's characteristics

Pen@25°C: 100 dmm

T_{R&B}: 60°C

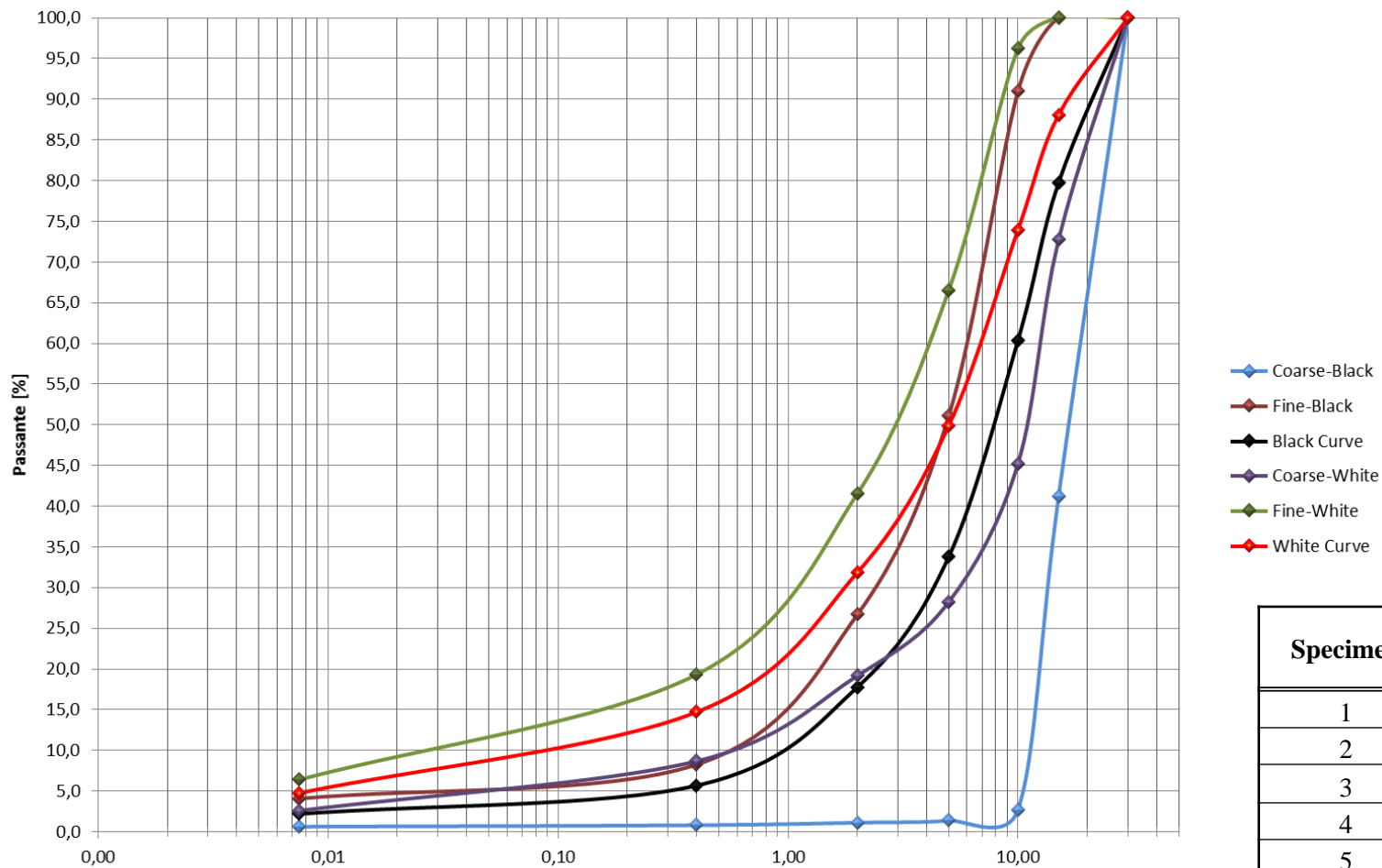
Elastic recovery: 75%

Bitumen for foaming

Pure bitumen

Pen@25°C: 70 -100

T_{R&B}: 51°C



Specimen	% bitumen
1	7,5
2	10,7
3	5,9
4	6,0
5	5,6
6	4,7
8	4,3
9	4,3

MIXTURES

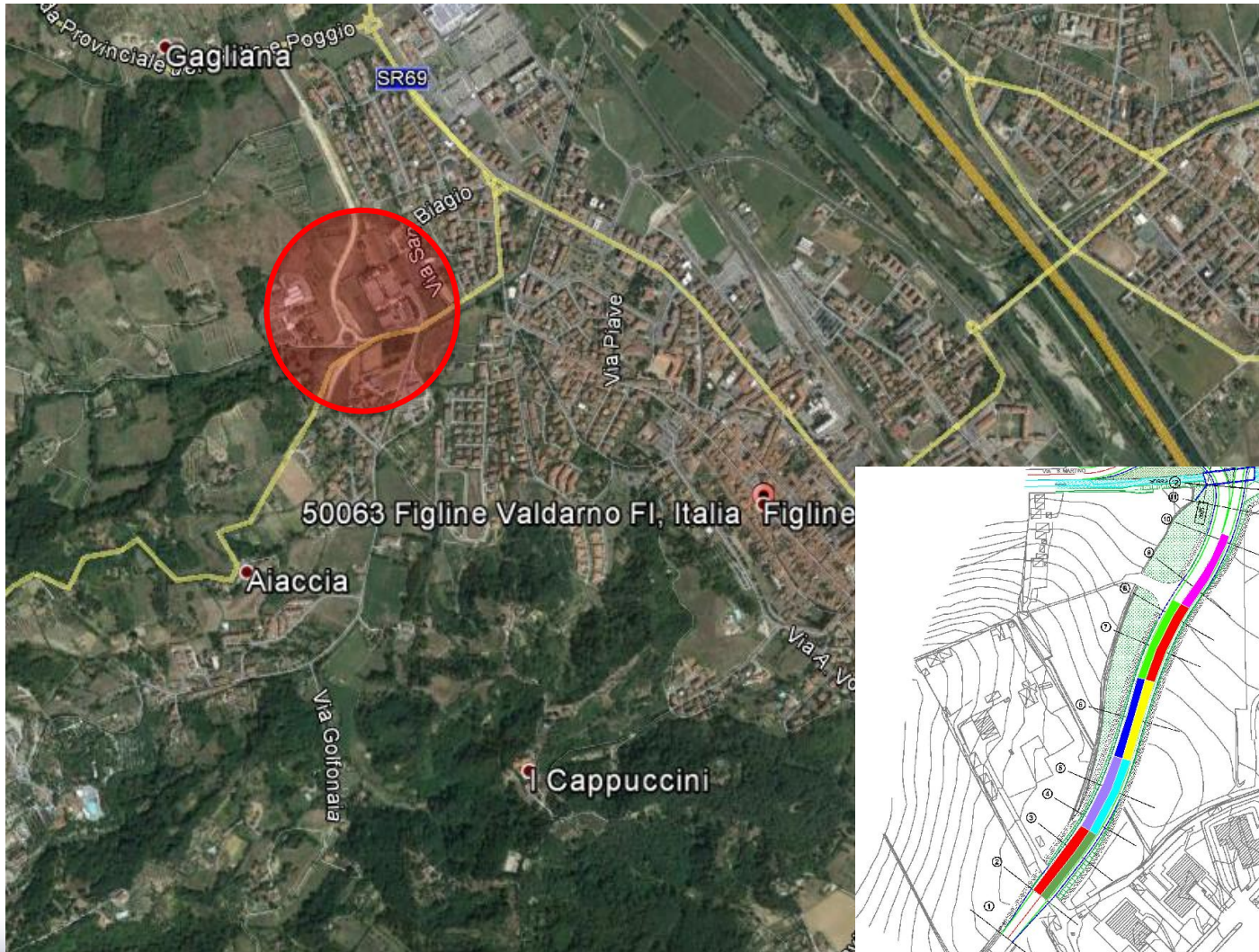


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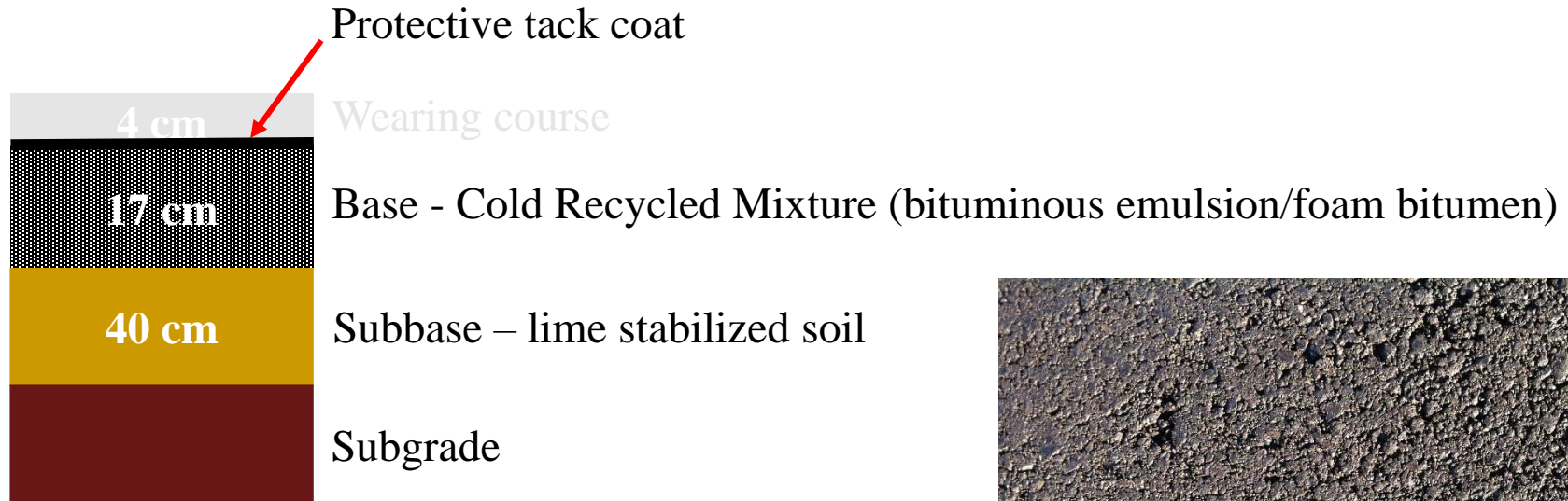
Total amount of filler: 4.5%

	Mix	%binder	%cement	%lime	%mineral filler
Emulsion	2A	3	1.0	2.0	1.5
	2B	3	1.0	0	3.5
	4D	3	2.5	0	2.0
Foam bitumen	3A	2	1.0	2.0	1.5
	3B	2	1.0	0	3.5
	5C	3	2.5	2.0	0
	5D	3	2.5	0	2.0
	5E	3	0	2.0	2.5
	5F	3	0	3.0	1.5

TRIAL SECTION - FLORENCE (ITALY)



TRIAL SECTION - FLORENCE (ITALY)



Thanks to a particular work plan the wearing coarse was layed down 10 months after wearing course

 **All mixtures had the curing process without traffic load**

 **First LWD/FWD test campaign directly on CRM layer**

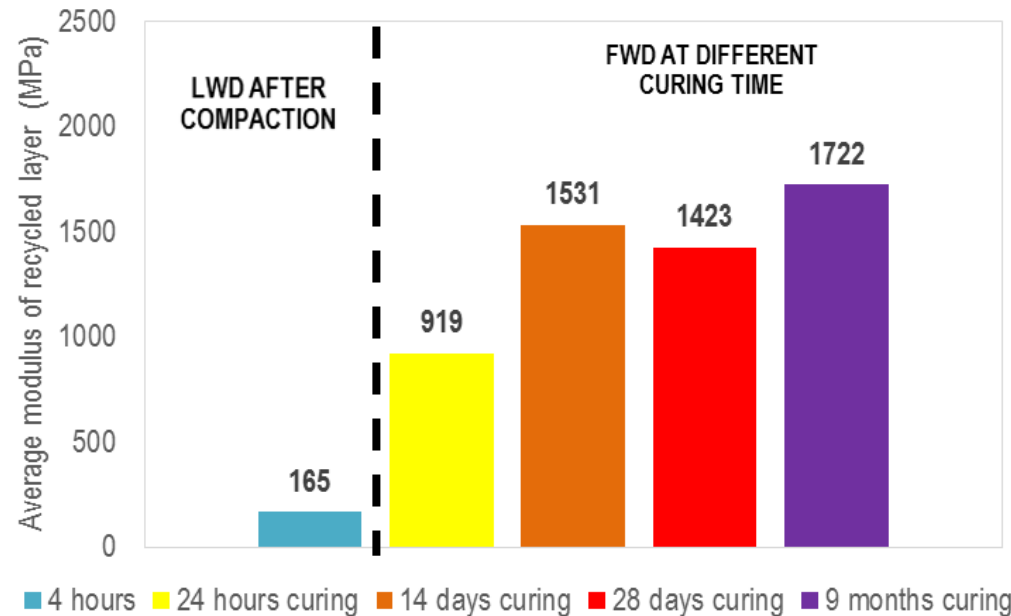
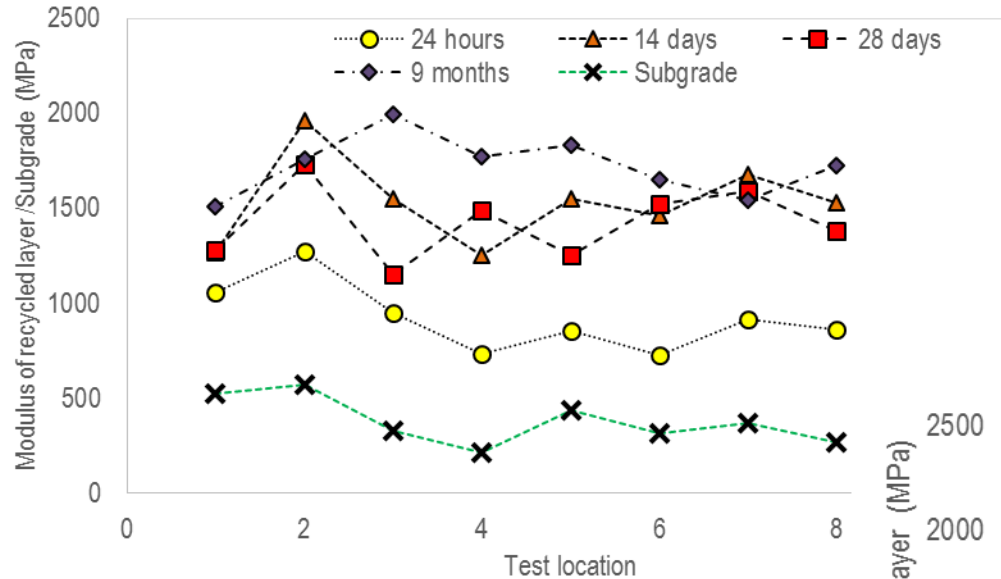


LWD & FWD TESTS RESULTS

MIX 5D_3%FB_2,5%C_0%L_2%MF



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Minimum requirement of Italian Road Authority (ANAS)

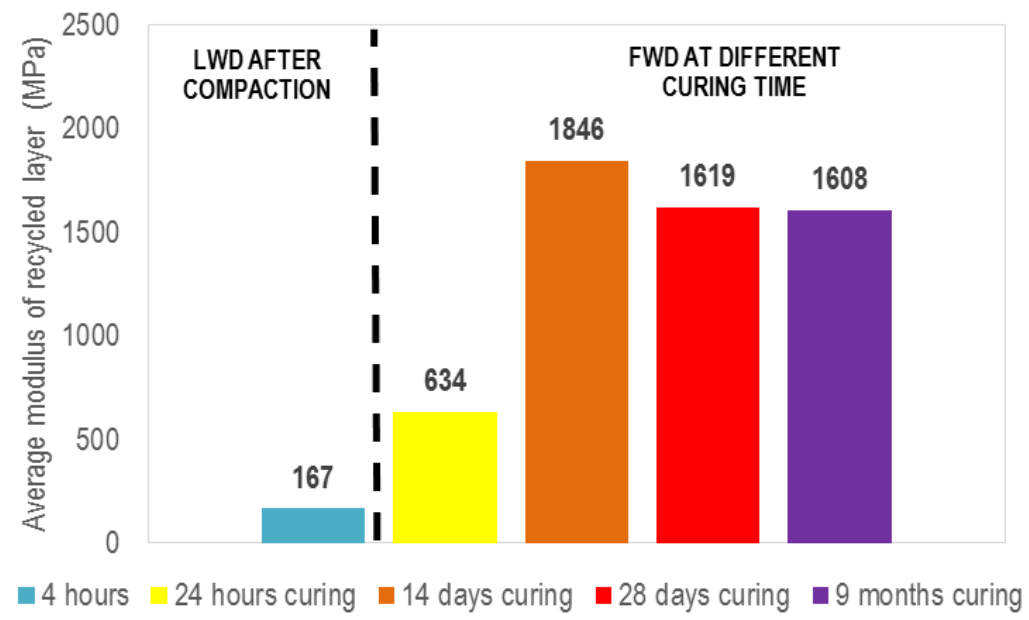
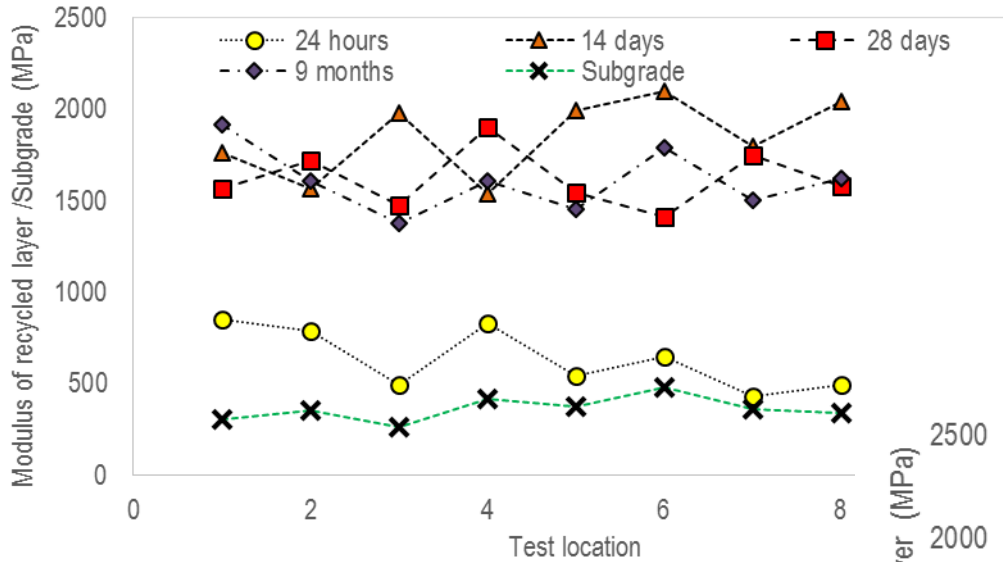
- 45 MPa 4 hours after compaction
- 170 MPa 24 hours after compaction

LWD & FWD TESTS RESULTS

MIX_5E_3%FB_0%C_2%L_2,5%MF



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Minimum requirement of Italian Road Authority (ANAS)

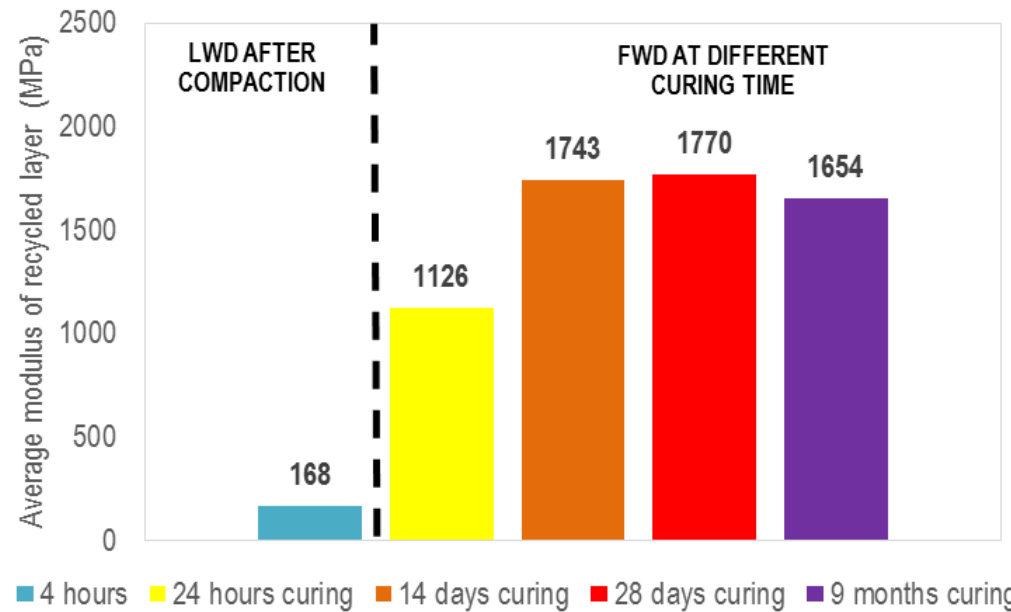
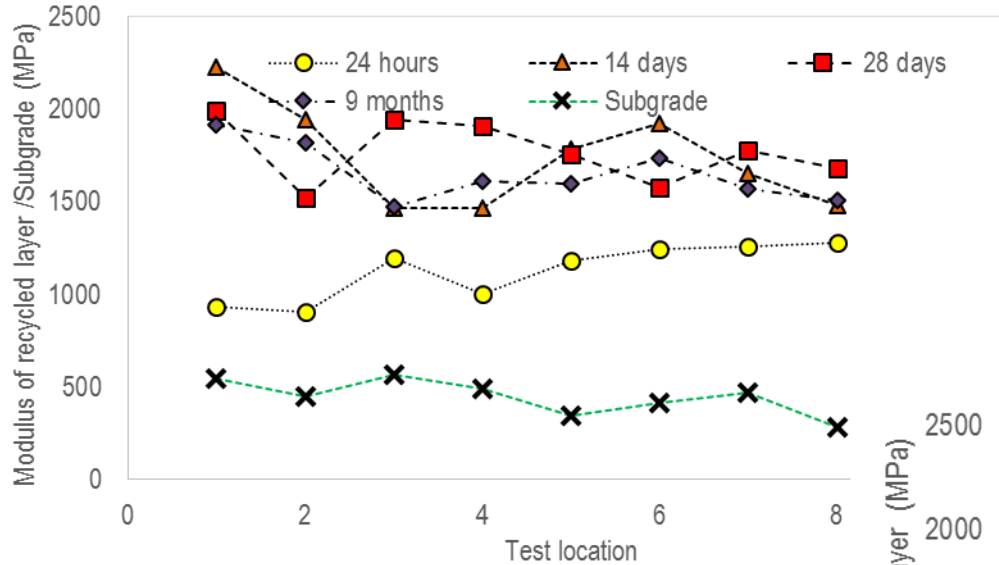
- 45 MPa 4 hours after compaction
- 170 MPa 24 hours after compaction

LWD & FWD TESTS RESULTS



MIX_3B_2%FB_1%C_0%L_3.5%MF

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Minimum requirement of Italian Road Authority (ANAS)

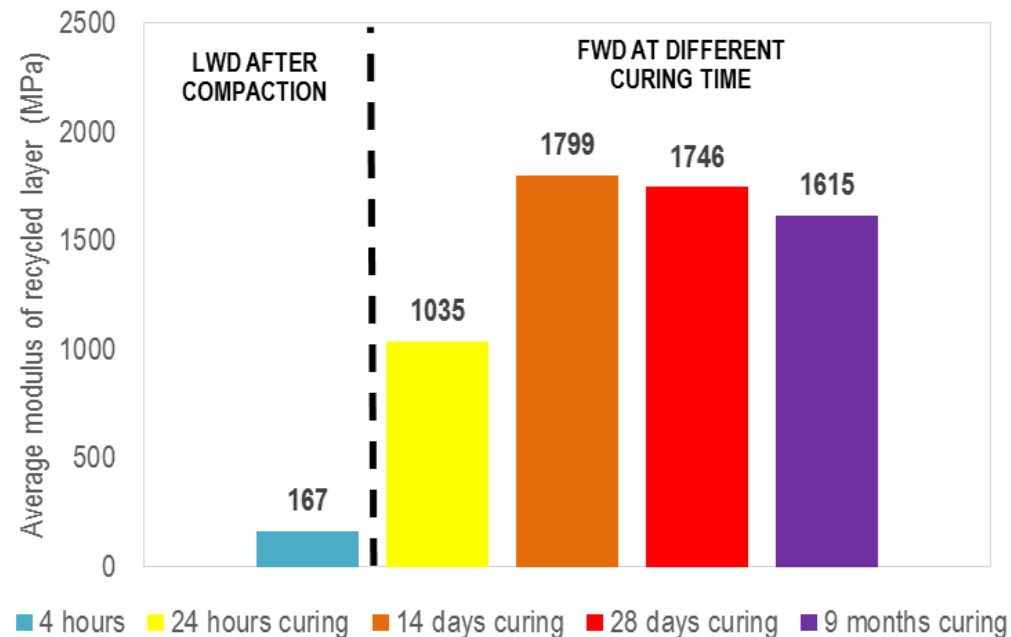
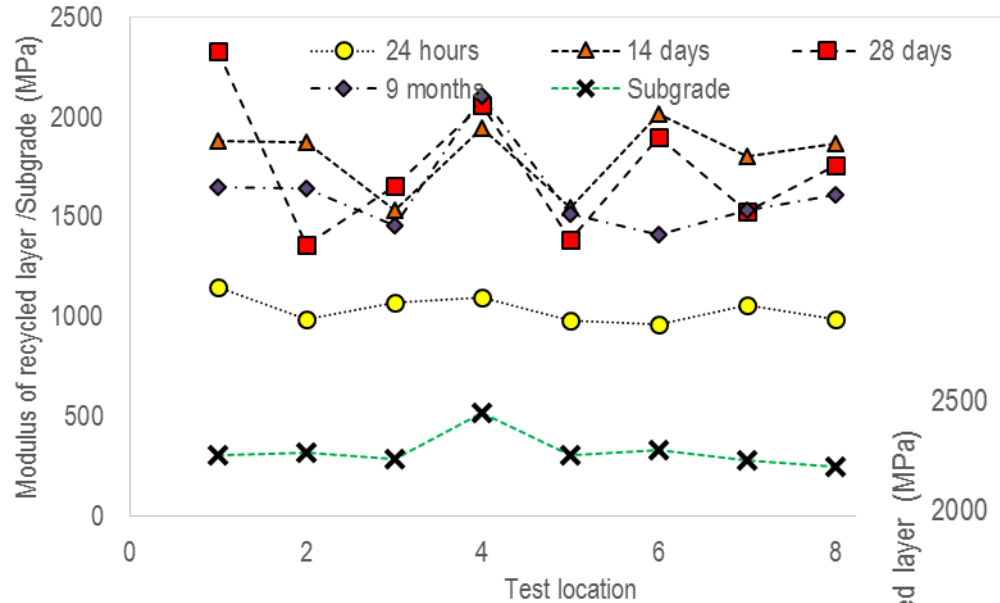
- 45 MPa 4 hours after compaction
- 170 MPa 24 hours after compaction

LWD & FWD TESTS RESULTS

MIX_3A_2%FB_1%C_2%L_1.5%MF



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Minimum requirement of

Italian Road Authority (ANAS)

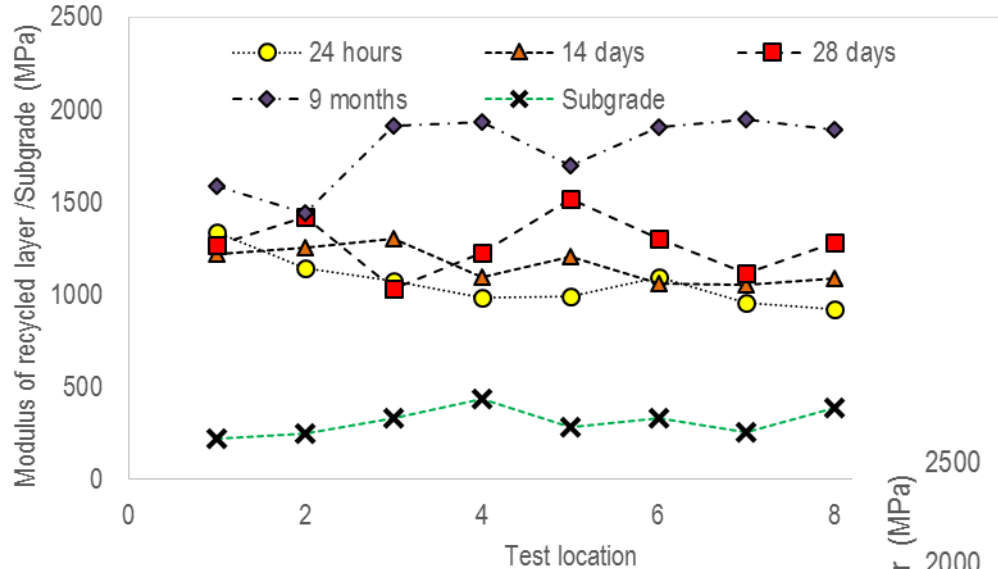
- **45 MPa 4 hours after compaction**
- **170 MPa 24 hours after compaction**

LWD & FWD TESTS RESULTS

MIX_5C_3%FB_2.5%C_2%L_0%MF

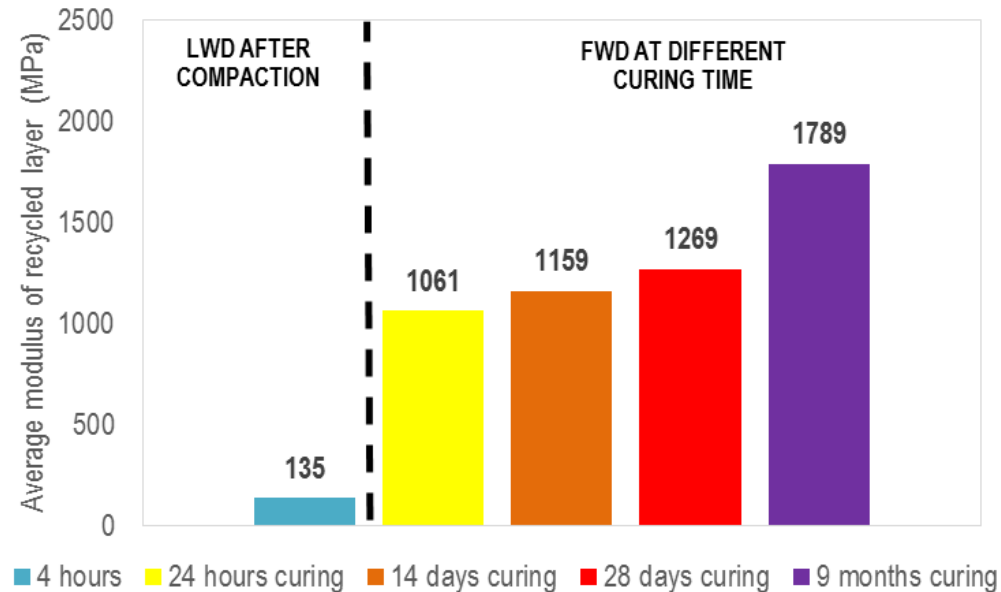


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Minimum requirement of Italian Road Authority (ANAS)

- 45 MPa 4 hours after compaction
- 170 MPa 24 hours after compaction

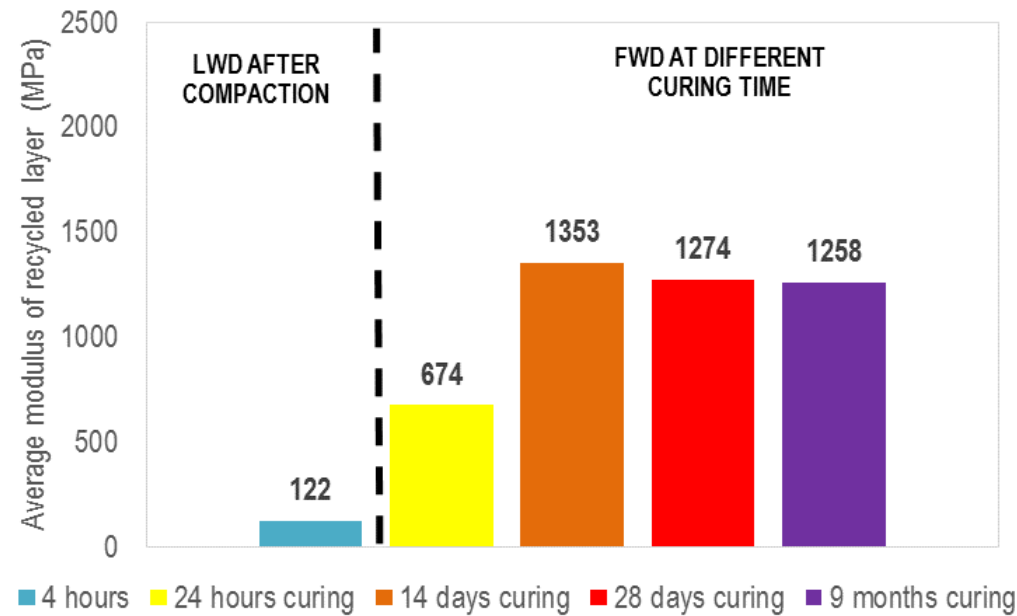
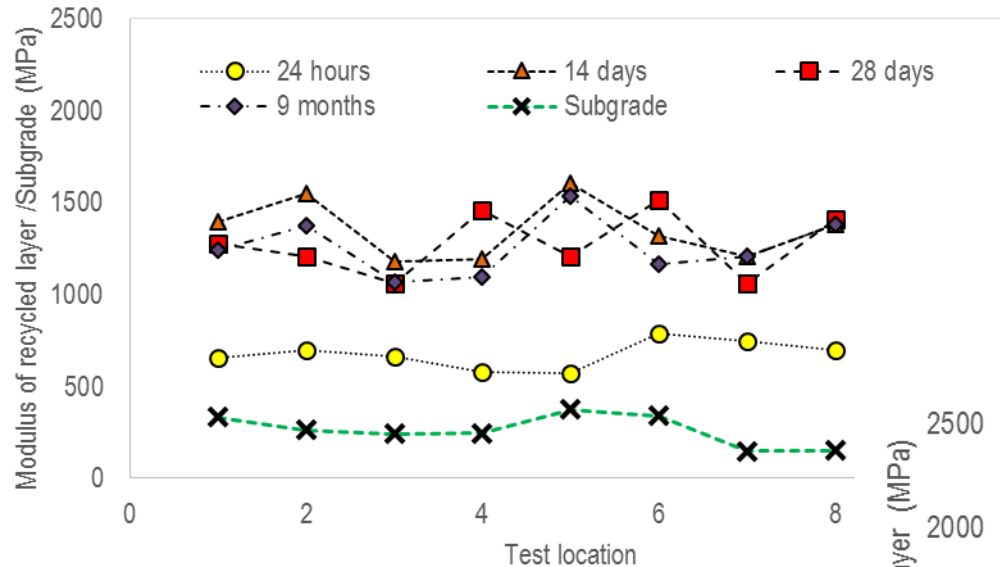


LWD & FWD TESTS RESULTS

MIX_5F_3%FB_0%C_3%L_1,5%MF



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Minimum requirement of

Italian Road Authority (ANAS)

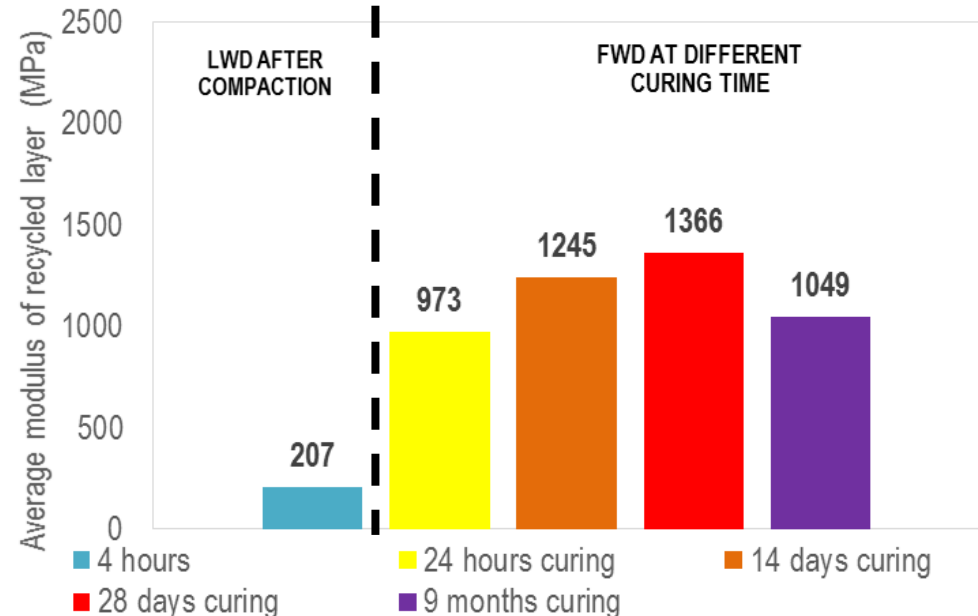
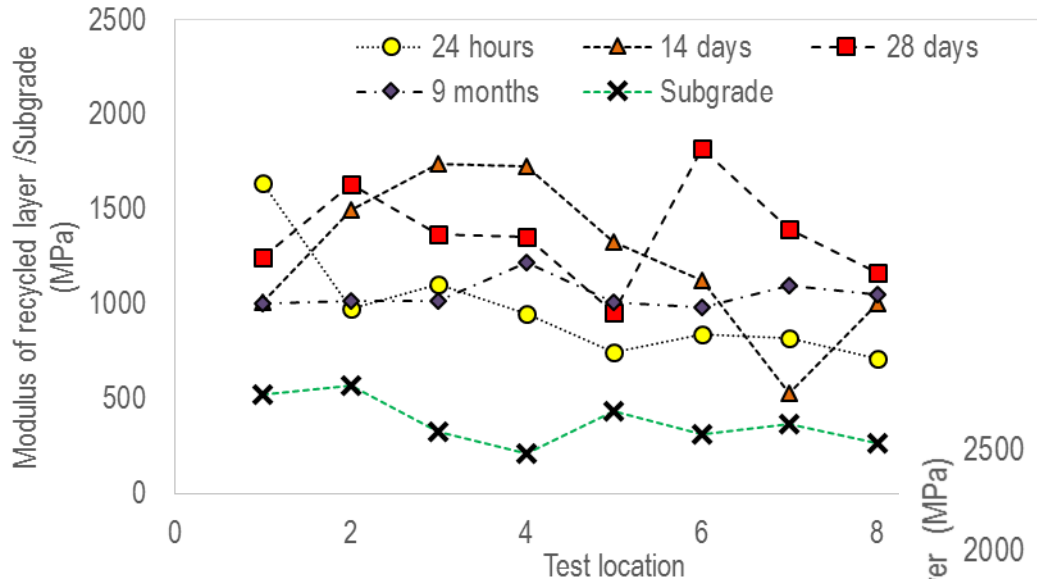
- **45 MPa 4 hours after compaction**
- **170 MPa 24 hours after compaction**

LWD & FWD TESTS RESULTS

MIX 2A 3%BE-1%C-2%L-1,5%MF



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Minimum requirement of Italian Road Authority (ANAS)

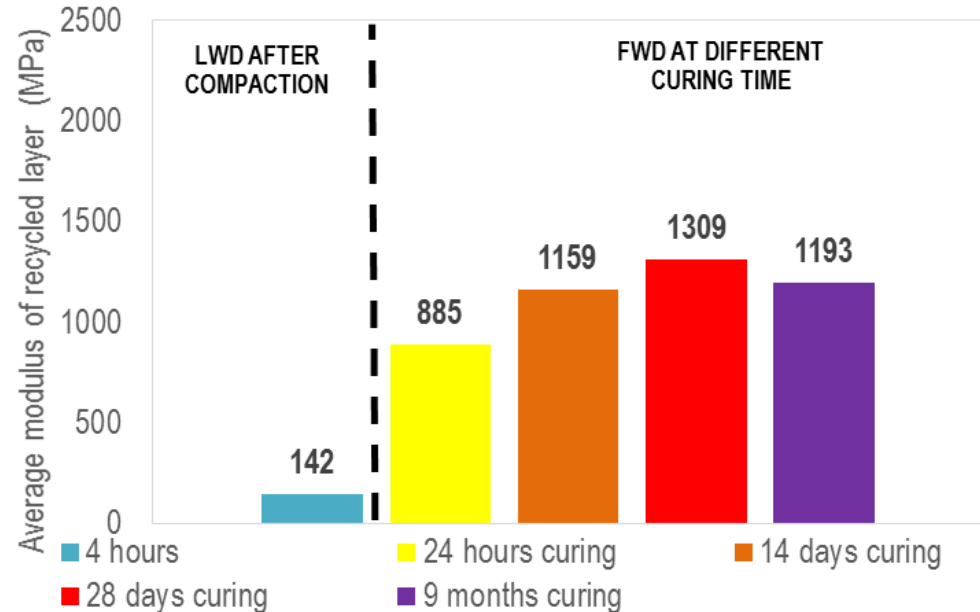
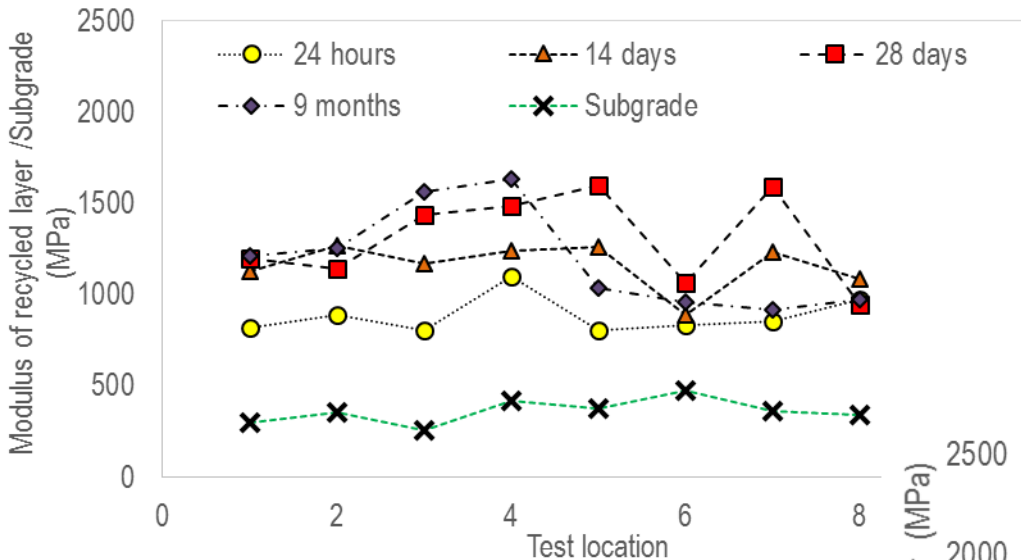
- 45 MPa 4 hours after compaction
- 170 MPa 24 hours after compaction

LWD & FWD TESTS RESULTS

Mix 2B 3%BE-1% C-0% L – 3.5% MF



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Minimum requirement of Italian Road Authority (ANAS)

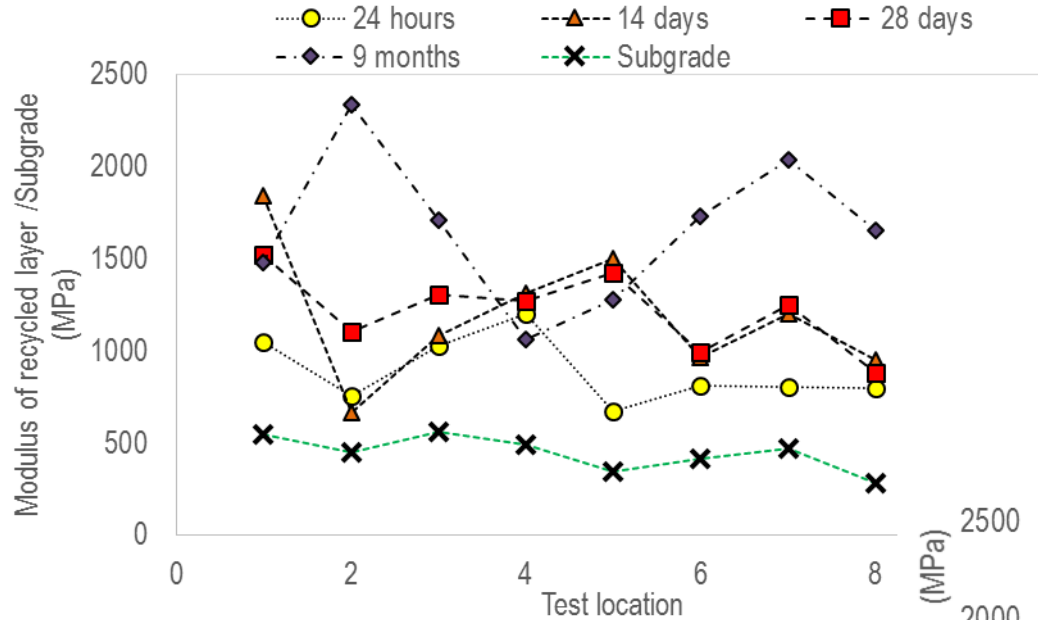
- 45 MPa 4 hours after compaction
- 170 MPa 24 hours after compaction

LWD & FWD TESTS RESULTS

Mix 4D 3%BE-2.5% C-0% L – 2% MF

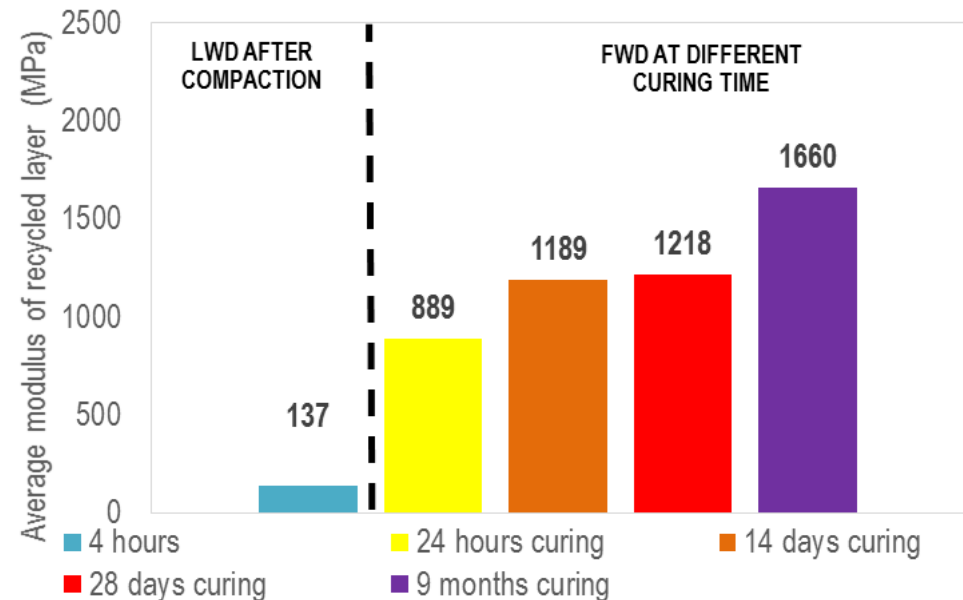


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Minimum requirement of Italian Road Authority (ANAS)

- 45 MPa 4 hours after compaction
- 170 MPa 24 hours after compaction

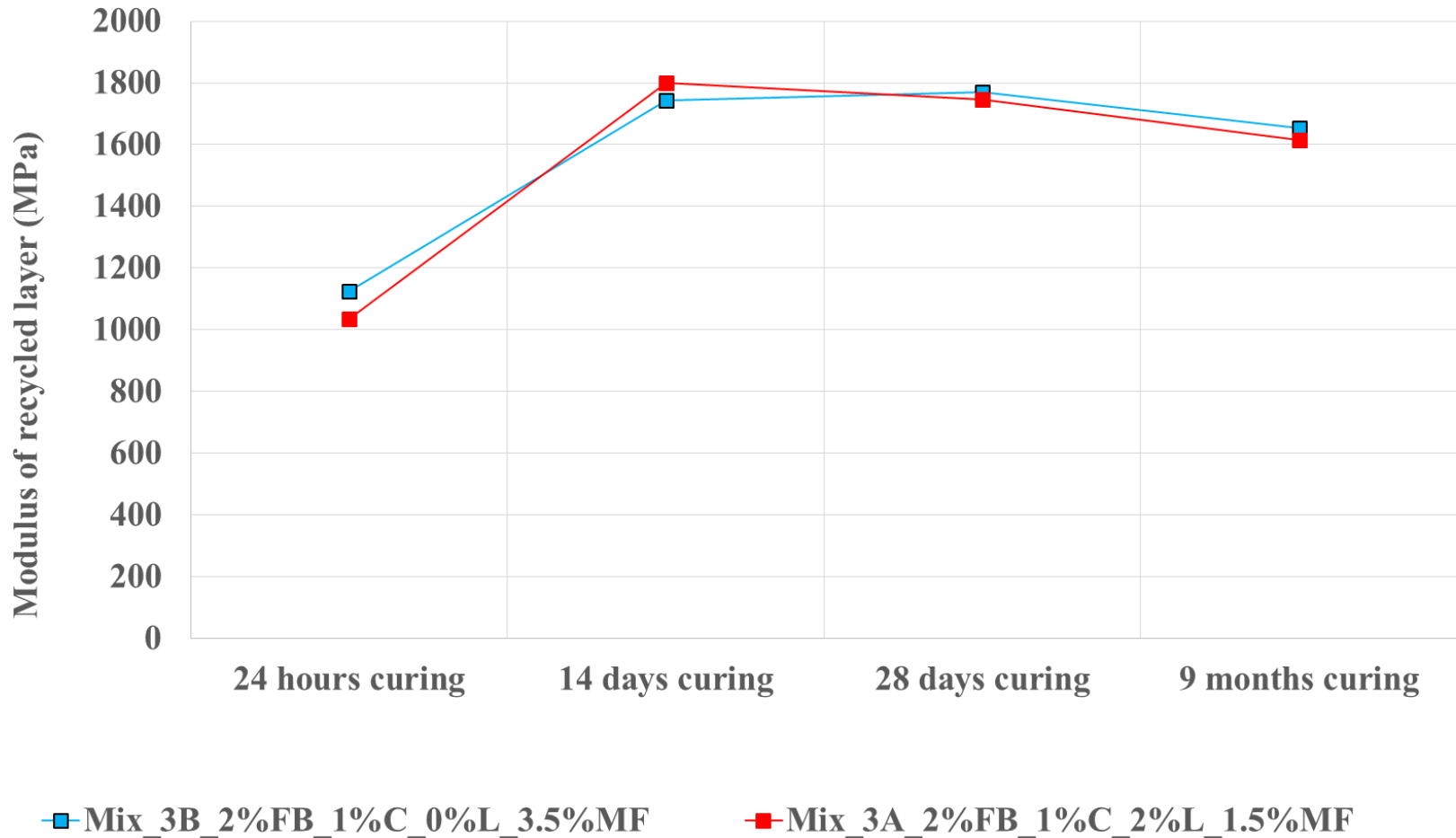


LWD & FWD TESTS RESULTS

LIME & CEMENT



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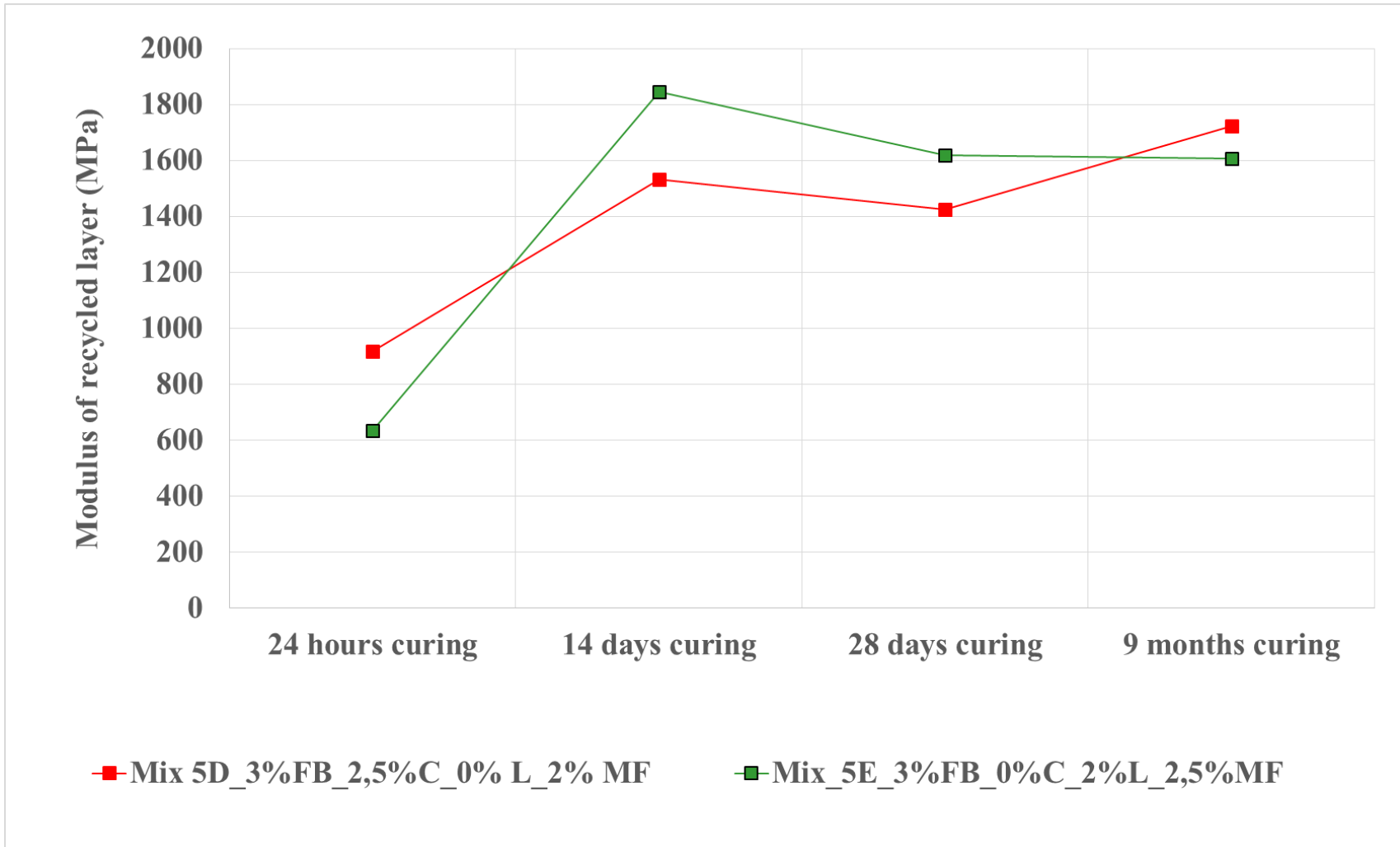


LWD & FWD TESTS RESULTS

LIME VS CEMENT

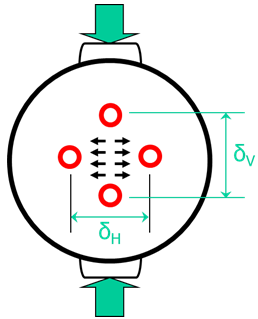


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FRACTURE ENERGY

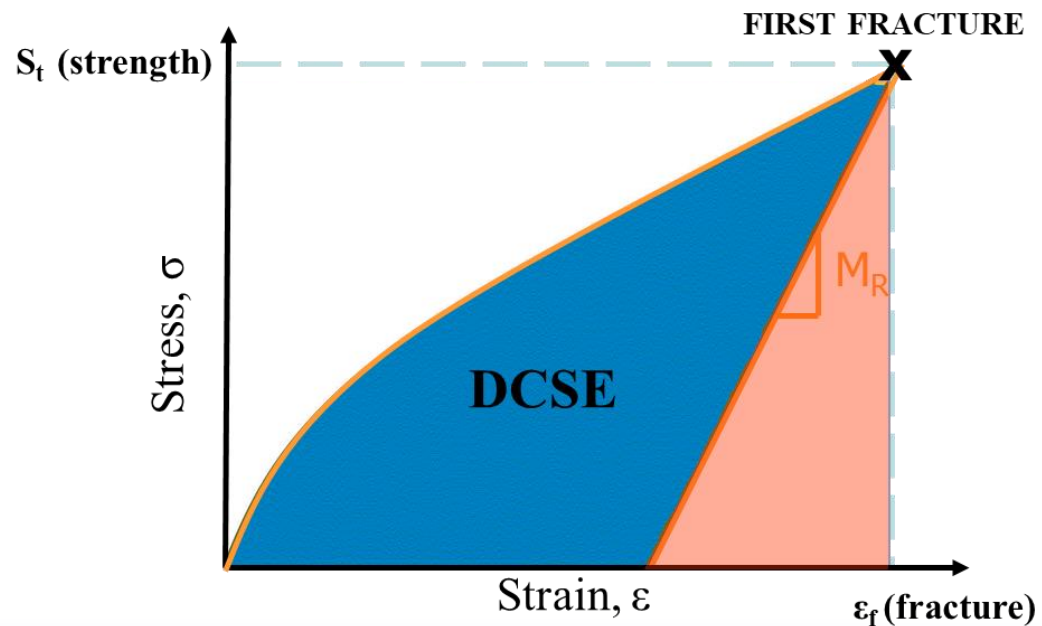
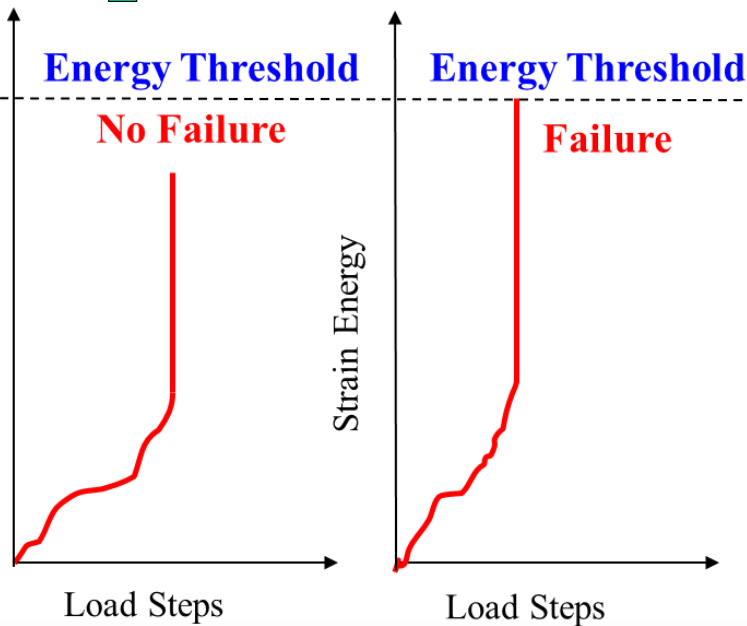
HMA Fracture Mechanics [Roque et al., 2011]



Total Fracture Energy (FE) \rightarrow creep + elastic

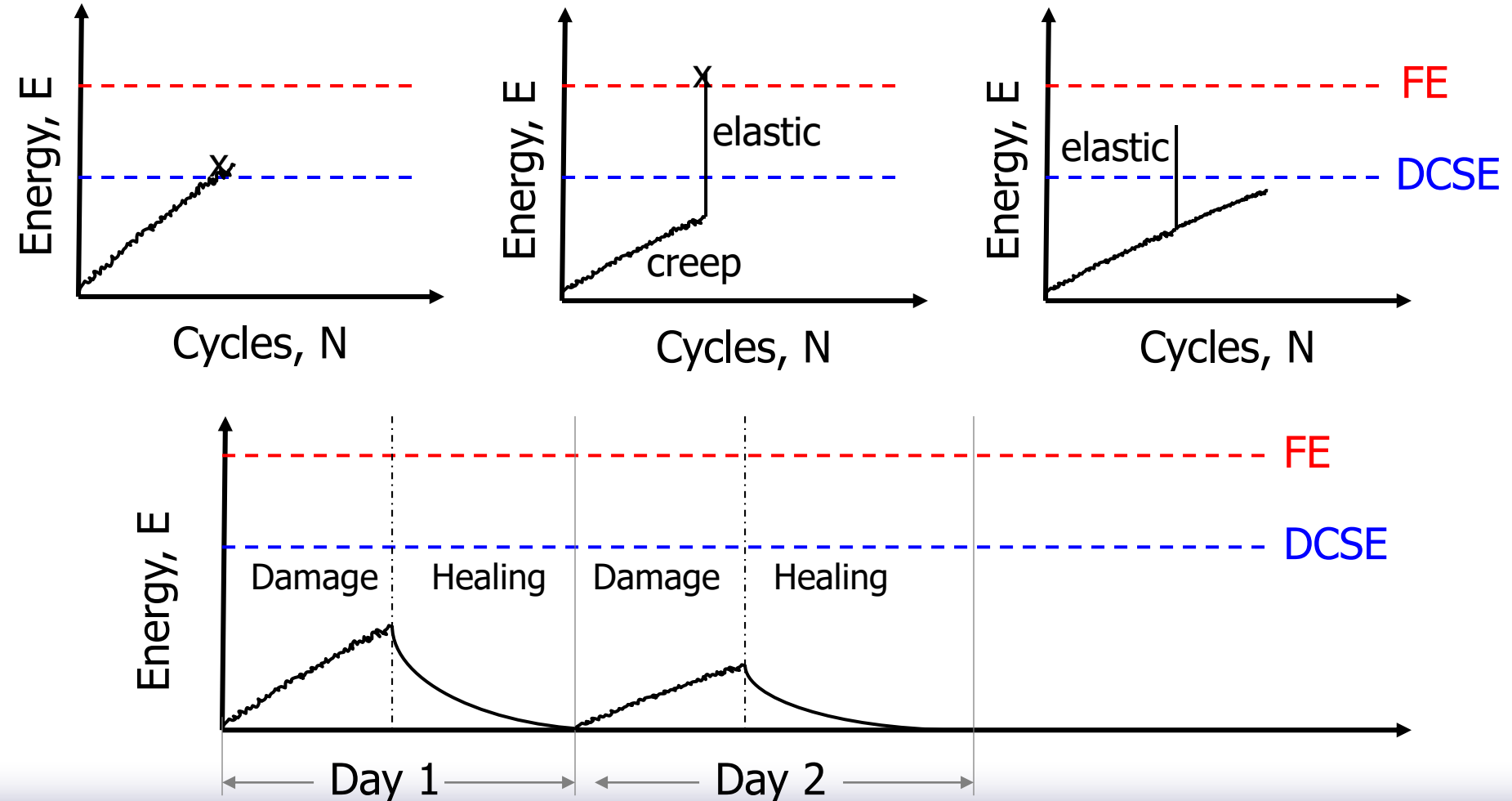
Creep Strain Energy (DCSE) \rightarrow creep only

$$FE = DCSE + EE$$



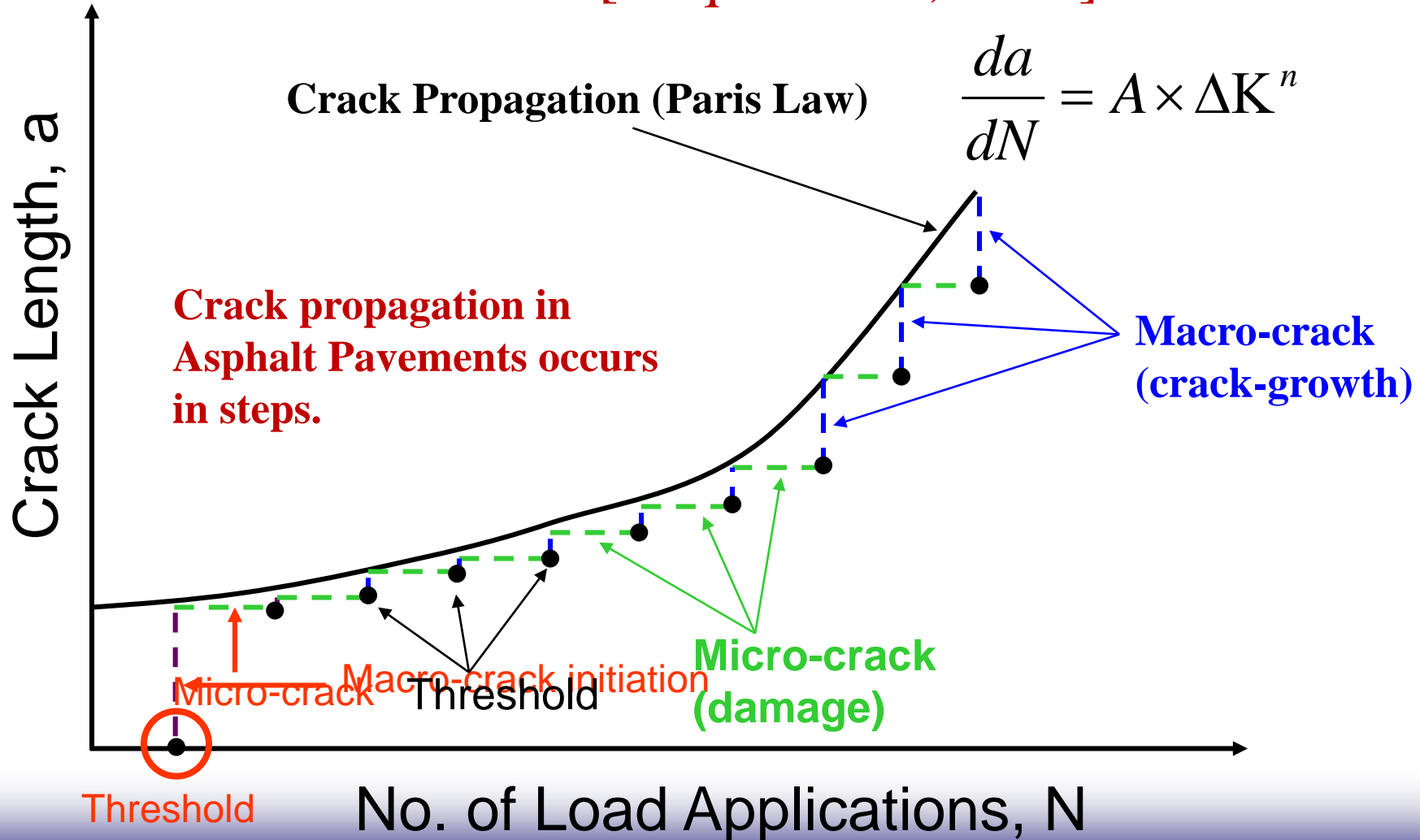
FRACTURE ENERGY

HMA Fracture Mechanics [Roque et al., 2011]



FRACTURE ENERGY

HMA Fracture Mechanics [Roque et al., 2011]

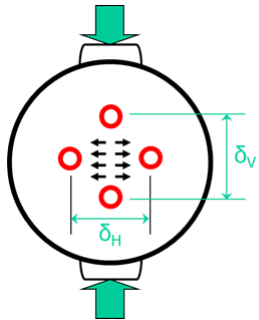
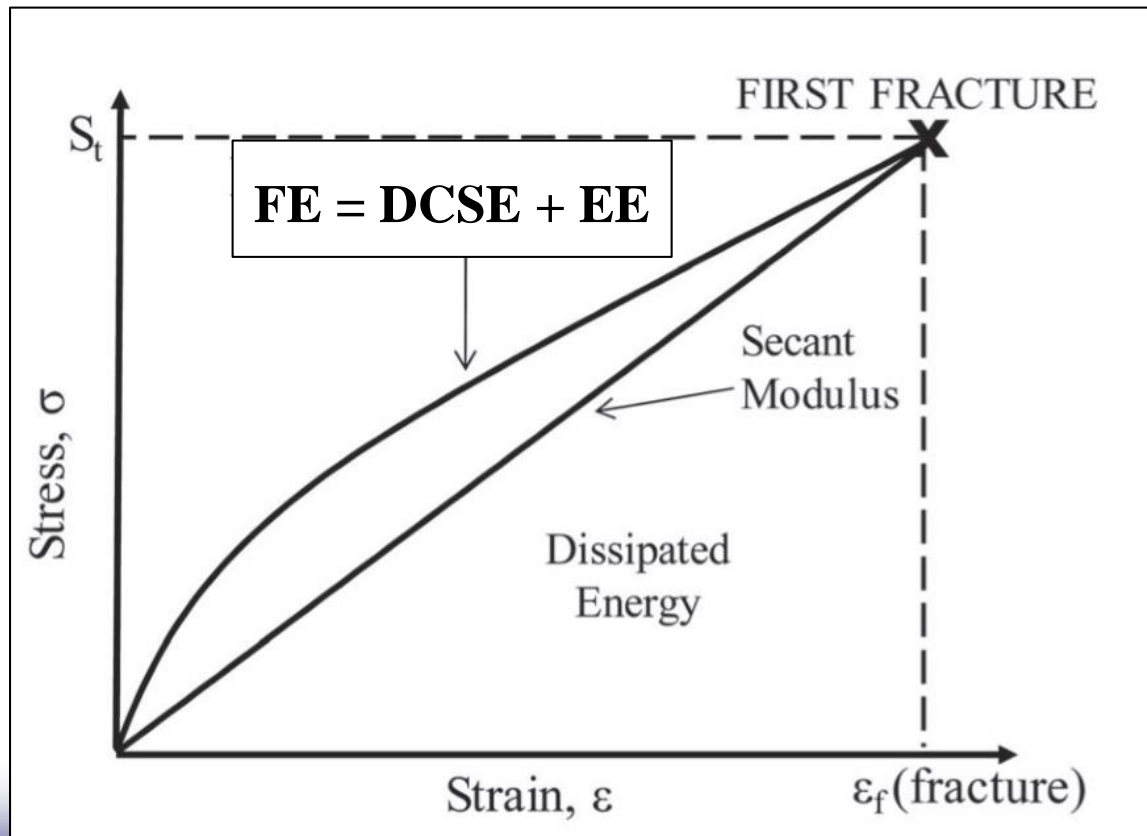


FRACTURE ENERGY

HMA Fracture Mechanics [Roque et al., 2011]

approach for quasi-brittle materials

Total Fracture Energy (FE) → dissipated energy + strain energy



FRACTURE ENERGY

Bituminous emulsion

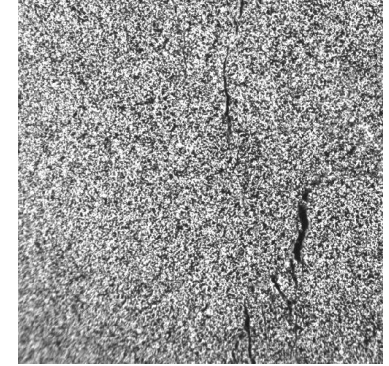
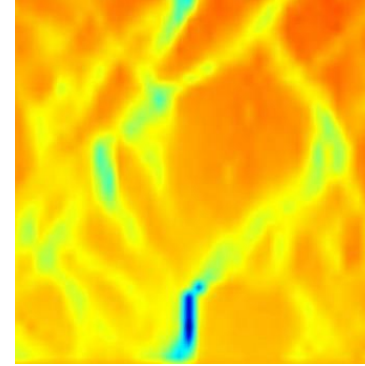
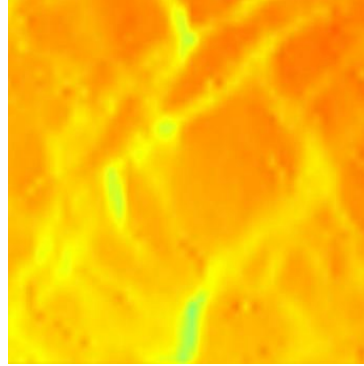


MIX	% bitumen emulsion	% cement	% hydrated lime	St [MPa]	Failure Strain [$\mu\epsilon$]	FE [Kj/m ³]	Dissipated Energy [Kj/m ³]	Strain Energy [Kj/m ³]
2A	3	1	2	0.30	910	0.29	0.136	0.153
2B	3	1	0	0.12	814	0.12	0.049	0.070
4D	3	2.5	0	0.21	571	0.18	0.061	0.117



HORIZONTAL

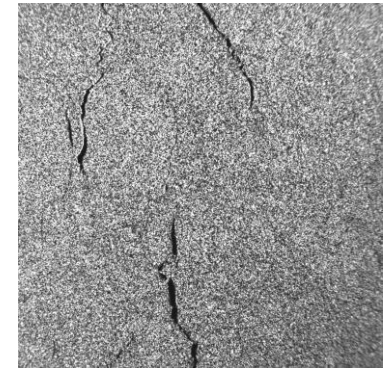
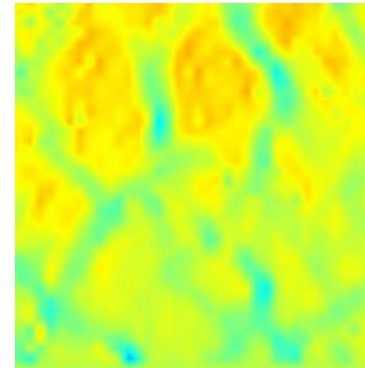
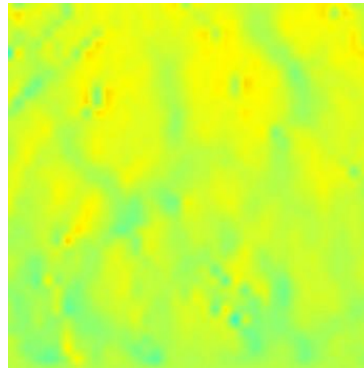
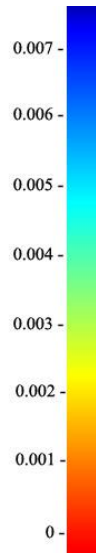
Bituminous emu



Mix 2A – macrocrack

Mix 2A – peak load

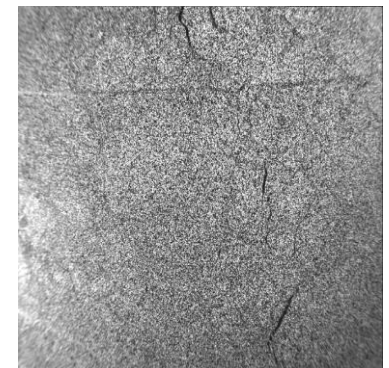
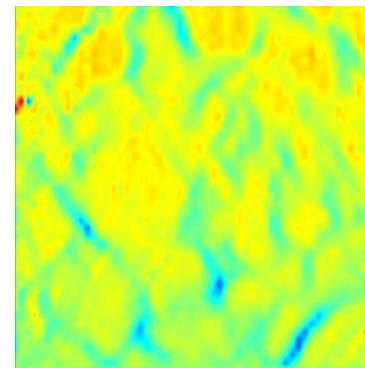
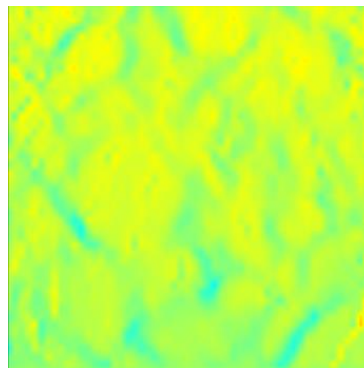
Mix 2A – visible cracks



Mix 2B – macrocrack

Mix 2B – peak load

Mix 2B – visible cracks



Mix 4D – macrocrack

Mix 4D – peak load

Mix 4D – visible cracks

MIX	% bitumen emulsion	% cement	% hydrated lime
2A	3	1	2
2B	3	1	0
4D	3	2.5	0


HORIZONTAL FULL FIELD STRAIN MAPS



University of Parma

Foam bitumen



MIX	% foam bitumen	% cement	% hydrated lime	St [MPa]	Failure Strain [$\mu\epsilon$]	FE [Kj/m^3]	Dissipated Energy [Kj/m^3]	Strain Energy [Kj/m^3]
3A	2	1	2		1624	0.81	0.261	0.552
3B	2	1	0	0.29	938	0.40	0.138	0.258
5C	3	2.5	2.0	0.29	702	0.61	0.103	0.504
5D	3	2.5	0	0.36	636	0.47	0.114	0.359
5E	3	0	2.0	0.19	1092	0.51	0.103	0.408
5F	3	0	3.0	0.23	1091	0.59	0.123	0.464



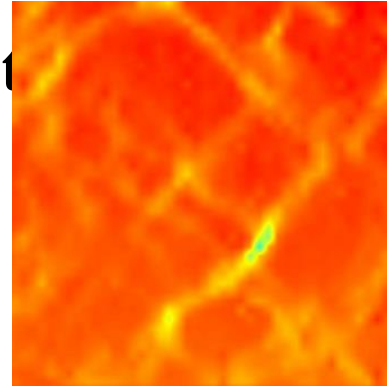
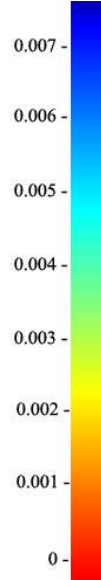
HORIZONTAL FULL FIELD STRAIN MAPS

Foam bitumen

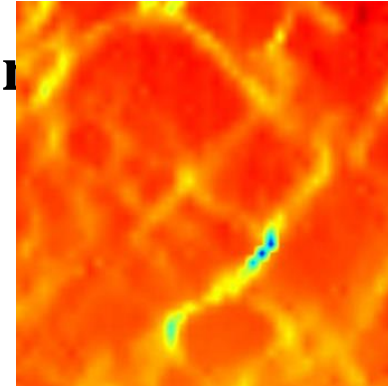
with lime

Strain

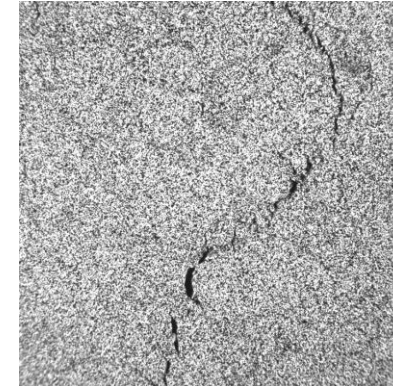
Crack



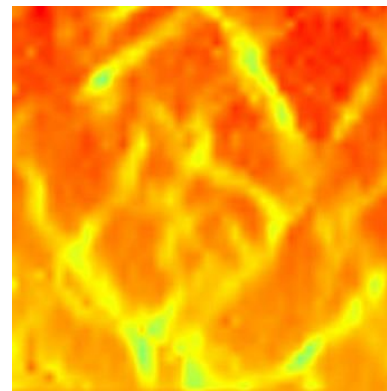
Mix 3A – macrocrack



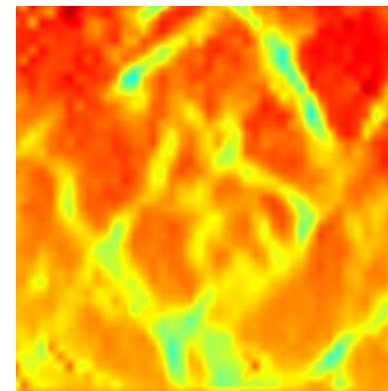
Mix 3A – peak load



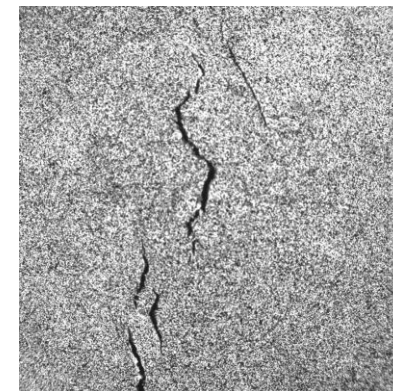
Mix 3A – visible cracks



Mix 5D – macrocrack



Mix 5D – peak load



Mix 5D – visible cracks

without lime

MIX	% foam bitumen	% cement	% hydrated lime
3A	2	1	2
3B	2	1	0
5C	3	2.5	2.0
5D	3	2.5	0
5E	3	0	2.0
5F	3	0	3.0

CONCLUSIONS



University of Parma

- Lime can be used instead of cement and together with cement as an active filler for bitumen stabilized materials
- Active fillers have limited influence on bearing capacity
- Active fillers have influence in the redistribution of the stress after crack initiation (*lime increases failure strain and fracture energy, cement reduce fracture energy*)
- Moderate amount of cement + lime may represent the optimal blending for bitumen stabilized mixtures

COLD RECYCLING & LIME

THANKS !!



QUESTIONS??